IMPERIAL COMMUNITY COLLEGE DISTRICT

IMPERIAL VALLEY COLLEGE

TENNIS COURT RENOVATION SHADES AND NEW LIGHTING

380 EAST ATEN RD. IMPERIAL, CA 92251 (760) 352-8320

BOARD OF SCHOOL TRUSTEES:

AREA 1 - HORTENSIA ARMENDARIZ AREA 4 - ISABEL SOLIS

AREA 7 - STEVEN M. TAYLOR

AREA 2 - KARLASIGMOND, BOARD PRESIDENT AREA 5 - MARK EDNEY

AREA 3 - JERRY HART

AREA 6 - ROMUALDO J. MEDINA, BOARD CLERK

DR. LENNOR M. JOHNSON, SUPERINTENDENT

SHEET INDEX KEY / KEY PLAN - DISCIPLINE (eq. A = ARCHITECTURAL, P = PLUMBING, etc) — DRAWING TYPE (eq. FLOOR PLAN, ARCHITECTURAL SECTIONS, etc.) - DRAWING TYPE SUB-SHEET NUMBER Á2.2 ARCHITECTURAL SECTIONS - DRAWING TYPE SUB-SHEET NUMBER

WESTMORLAND -

STATEMENT OF GENERAL CONFORMANCE ☐ THE DRAWINGS OR SHEETS LISTED ON THE COVER OR INDEX SHEET ☐ THIS DRAWING, OR PAGE OF SPECIFICATIONS / CALCULATIONS FOR LIGHTS STANDARDS/SHADE STRUCTURES: LIGHT STANDARDS/SHADE STRUCTURES SECTION OF SHEET INDEX HAVE BEEN PREPARED BY OTHER DESIGN PROFESSIONALS OR CONSULTANTS WHO ARE LICENSED AND / OR AUTHORIZED TO PREPARE SUCH DRAWLINGS IN THIS STATE. IT HAS BEEN EXAMINED BY ME FOR: I) DESIGN CONTENT AND APPEARS TO MEET THE APPROPRIATE REQUIREMENTS OF TITLE24, CALIFORNIA CODE OF REGULATIONS AND THE PROJECT SPECIFICATIONS PREPARED BY ME, AND 2) COORDINATION WITH MY PLANS AND SPECIFICATIONS AND IS ACCEPTABLE FOR INCORPORATION INTO THE CONSTRUCTION OF THIS PROJECT. THE STATEMENT OF GENERAL CONFORMANCE "SHALL NOT BE CONSTRUED AS RELIEVING ME OF MY RIGHTS, DUTIES, AND RESPONSIBILITIES UNDER SECTIONS 17302 AND 81138 OF THE EDUCATION CODE AND SECTIONS 4-336, 4-341 AND 4-344" OF (TITLE 24, PART I, SECTION 4-317 (b)) ☐ ALL DRAWINGS OR SHEETS LISTED ON THE COVER OR INDEX SHEET ☐ THIS DRAWING OR PAGE OF SPECIFICATIONS / CALCULATIONS FOR ☐ IS/ARE IN GENERAL CONFORMANCE WITH THE PROJECT DESIGN, AND ☐ HAS/HAVE BEEN COORDINATED WITH THE PROJECT PLANS

SOILS REPORT PRIOR TO BID SUBMITTAL. COPIES OF REPORT ARE AVAILABLE

ALL WORK SHALL CONFORM TO 2019 TITLE 24, CALIFORNIA CODE OF REGULATIONS (CCR).

(2018 IAPMO UNIFORM MECHANICAL CODE AND 2019 CALIFORNIA AMENDMENTS)

(2018 INTERNATIONAL BUILDING CODE, VOL. 1 & 2, AND 2019 CALIFORNIA

(2017 NATIONAL ELECTRICAL CODE AND 2019 CALIFORNIA AMENDMENTS)

(2018 IAPMO UNIFORM PLUMBING CODE AND 2019 CALIFORNIA AMENDMENTS)

(2018 INTERNATIONAL FIRE CODE AND 2019 CALIFORNIA AMENDMENTS) 2019 CALIFORNIA EXISTING BUILDING CODE (CEBC), PART 10, TITLE 24 CCR

2016 ASME AIT.I/CSA B44-13 SAFETY CODE FOR ELEVATORS AND ESCALATORS

(2018 INTERNATIONAL EXISTING BUILDING CODE AND 2019 CALIFORNIA AMENDMENTS)

2019 CALIFORNIA GREEN BUILDING STANDARDS CODE (CALGREEN), PART II, TITLE 24 CCR 2019 CALIFORNIA REFERENCED STANDARDS CODE, PART 12, TITLE 24 CCR

2022 CALIFORNIA ADMINISTRATIVE CODE (CAC), PART I, TITLE 24, CCR

UPON REQUEST AND NOT LESS THAN 7 DAYS PRIOR TO BID DATE.

2019 CALIFORNIA BUILDING CODE (CBC), PART 2, TITLE 24 CCR

2019 CALIFORNIA ELECTRICAL CODE (CEC), PART 3, TITLE 24 CCR

2019 CALIFORNIA MECHANICAL CODE (CMC), PART 4, TITLE 24 CCR

2019 CALIFORNIA PLUMBING CODE (CPC), PART 5, TITLE 24 CCR

TITLE 19 CCR PUBLIC SAFETY STATE FIRE MARSHAL REGULATIONS

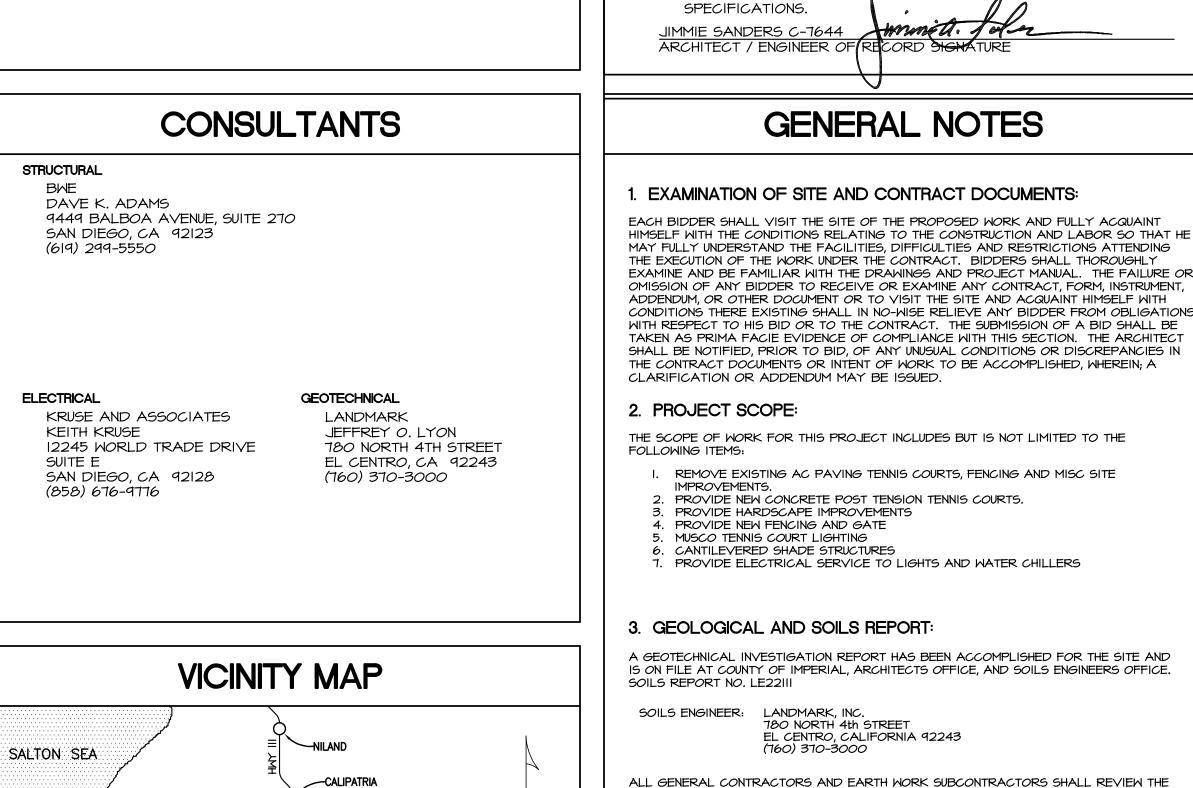
2019 CALIFORNIA ENERGY CODE (CEC), PART 6, TITLE 24 CCR

2019 CALIFORNIA FIRE CODE (CFC), PART 9, TITLE 24 CCR

4. CODES AND STANDARDS:

(PER 2019 CBC PART 2 CH 35)

APPLICABLE CODES:



UNITED STATES CALIFORNIA

MEXICO BAJA CALIFORNIA

	/OSHA ELEVATOR UNIT ENFORCES CCR TITLE & AND USES T : AI7.1 BY ADOPTION	
PARTIAL I	ist of applicable standards:	
NFPA 13		2016 EDITION
NFPA 14	SPRINKLER SYSTEMS (CA AMENDED) STANDARD FOR THE INSTALLATION OF STANDPIPE	2016 EDITION
NEPA 17	AND HOSE SYSTEMS (CA AMENDED) STANDARD FOR DRY CHEMICAL EXTINGUISHING	2017 EDITION
	SYSTEMS	
NFPA I7A	STANDARD FOR WET CHEMICAL EXTINGUISHING SYSTEMS	2017 EDITION
NFPA 20	STANDARD FOR THE INSTALLATION OF STATIONARY	2016 EDITION
NFPA 22	PUMPS FOR FIRE PROTECTION STANDARD FOR WATER TANKS FOR PRIVATE	2013 EDITION
NEDA 24	FIRE PROTECTION	2016 EDITION
	STANDARD FOR THE INSTALLATION OF PRIVATE FIRE SERVICE MAINS AND THEIR APPURTENANCES (CA AMENDED)	
	NATIONAL FIRE ALARM AND SIGNALING CODE (CA AMENDED) STANDARD FOR FIRE DOORS AND OTHER OPENING	
	PROTECTIVES	
NFPA 2001	STANDARD ON CLEAN AGENT FIRE EXTINGUISHING SYSTEMS (CA AMENDED)	
UL 300	STANDARD FOR FIRE TESTING OF FIRE EXTINGUISHING SYSTEMS FOUR PROTECTION OF	2005 EDITION
	EXTINGUISHING SYSTEMS FOR PROTECTION OF COMMERCIAL COOKING EQUIPMENT	(R2010)
UL 464	AUDIBLE SIGNALING DEVICES FOR FIRE ALARM	2003 EDITION
UL 521	AND SIGNALING SYSTEMS, INCLUDING ACCESSORIES STANDARD FOR HEAT DETECTORS FOR FIRE	1999 EDITION
	PROTECTIVE SIGNALING SYSTEMS	
UL 1971	STANDARD FOR SIGNALING DEVICES FOR THE HEARING IMPAIRED	(P2010
ICC 300	STANDARD FOR BLEACHERS, FOLDING AND	2017 EDITION
	TELESCOPIC SEATING, AND GRANDSTANDS	
SEE CALIFO	5 AND CALIFORNIA FIRE CODE CHAPTER 80. PRNIA BUILDING CODE CHAPTER 35 FOR STATE OF CALIFOR TS TO THE NFPA STANDARDS.	NIA
5. TESTI	NG AND INSPECTION:	
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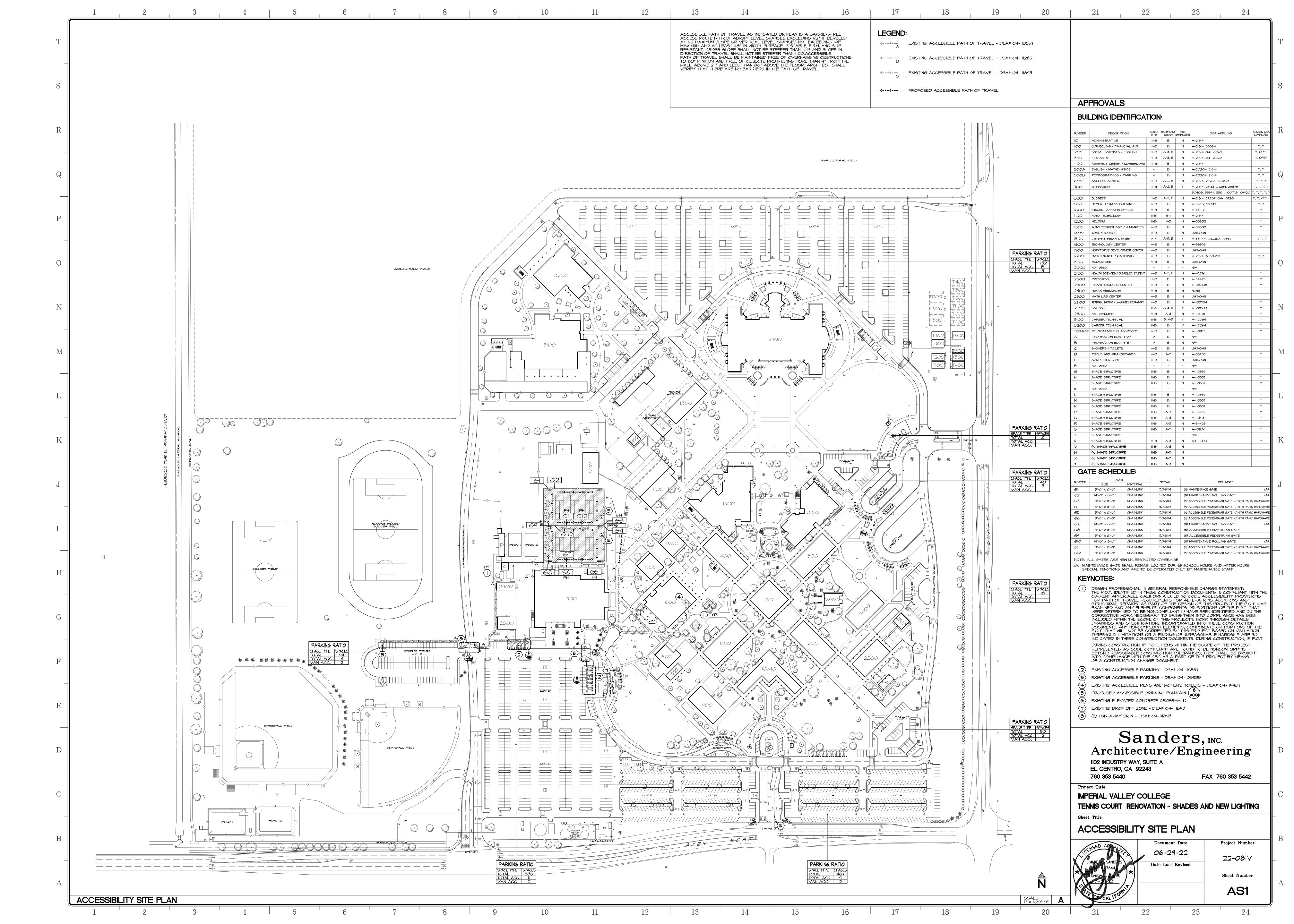
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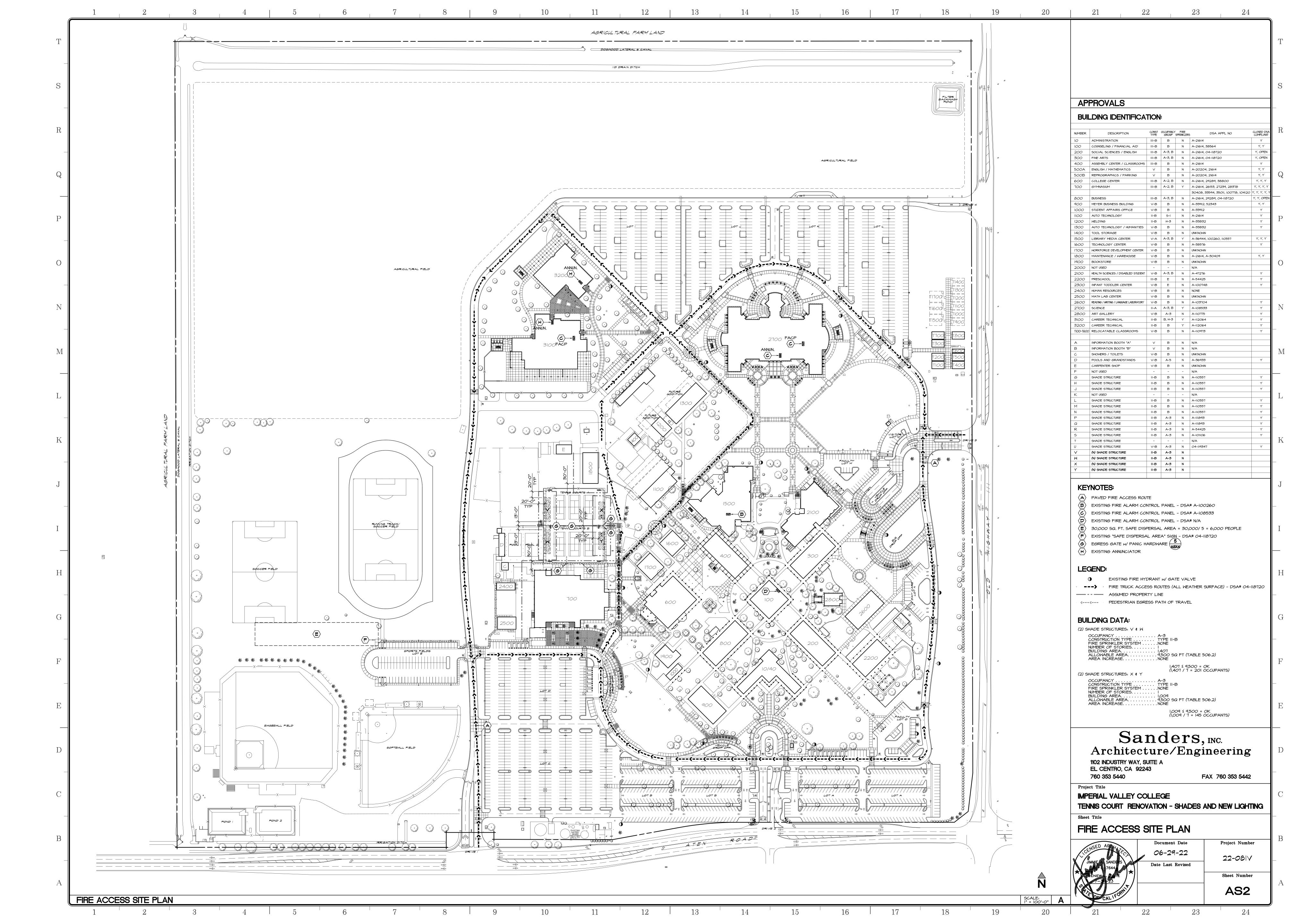
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OVIDE ELECTRICAL SERVICE TO LIGHTS AND WATER CHILLERS	AS4	SITE GRADING PLAN
BID EXCLUSIONS:	AS5	HARDSCAPE PLAN
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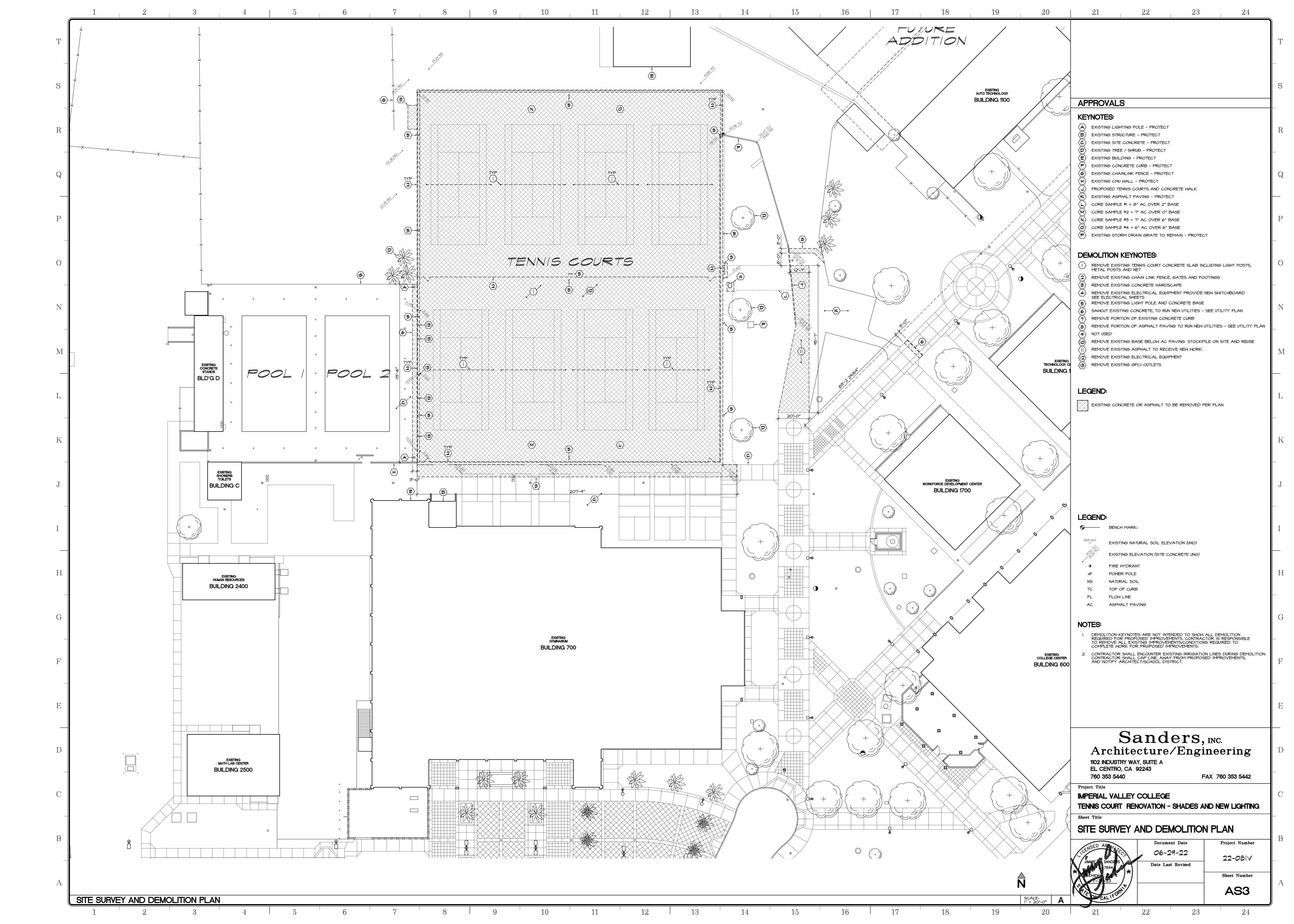
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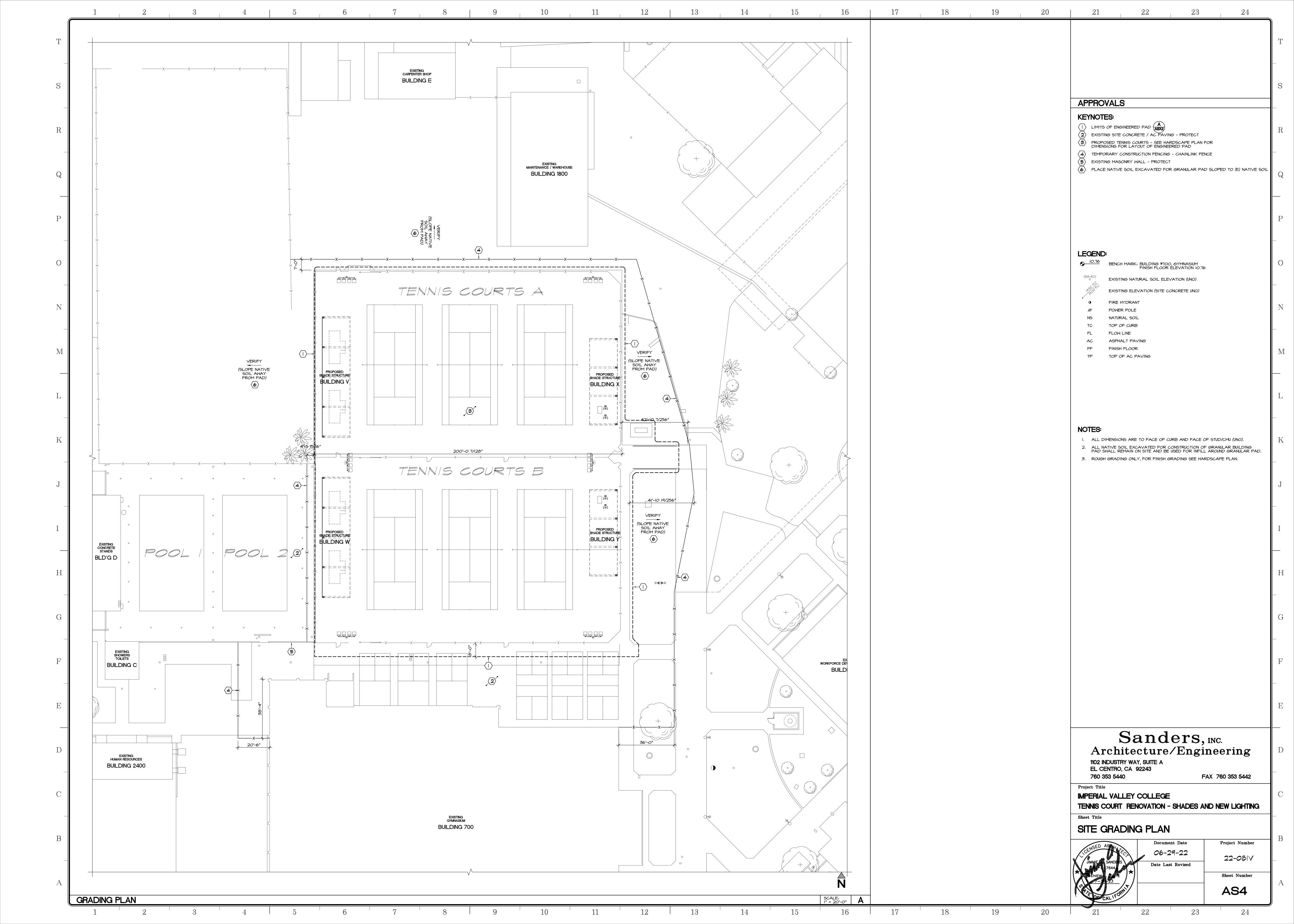
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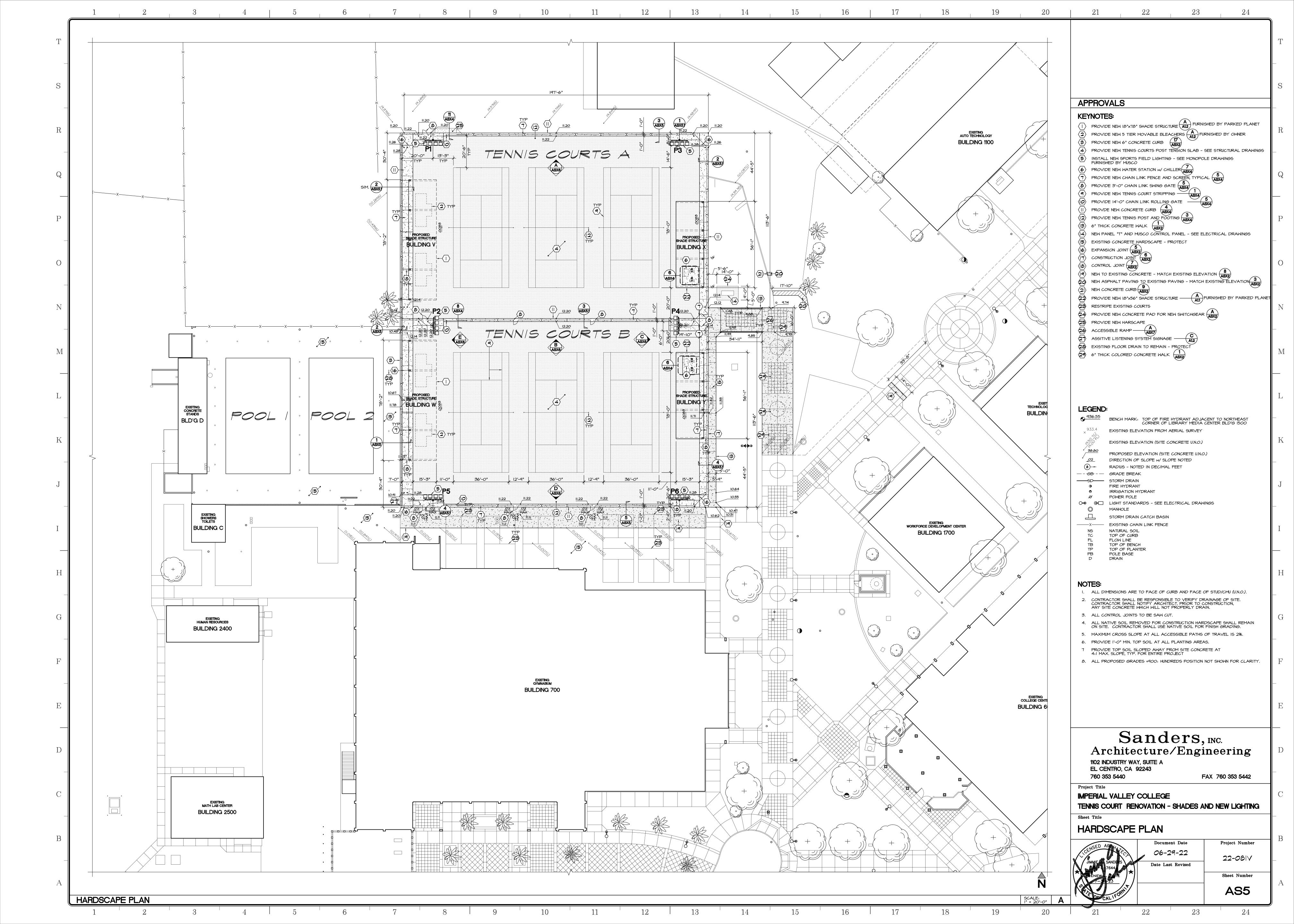
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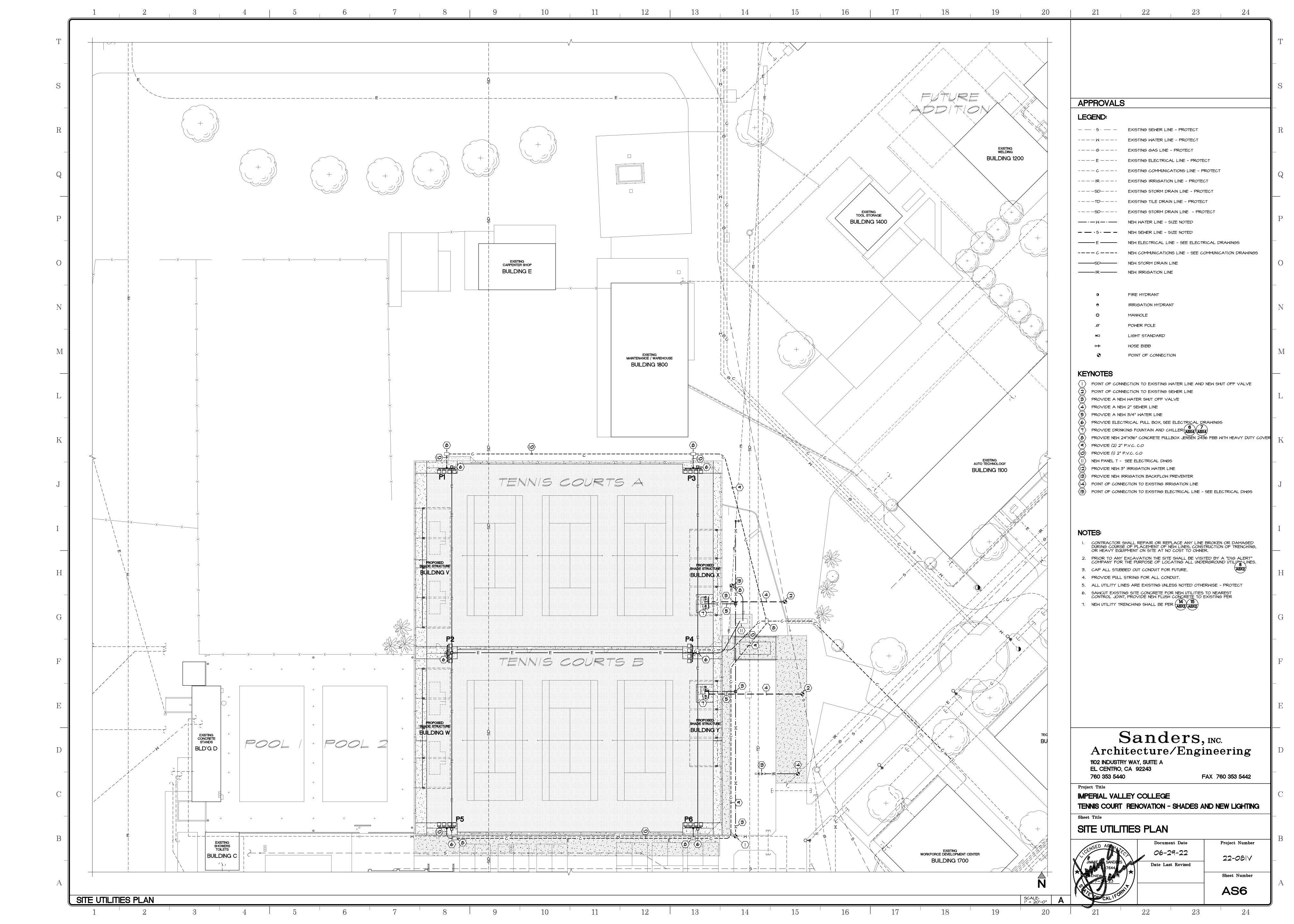


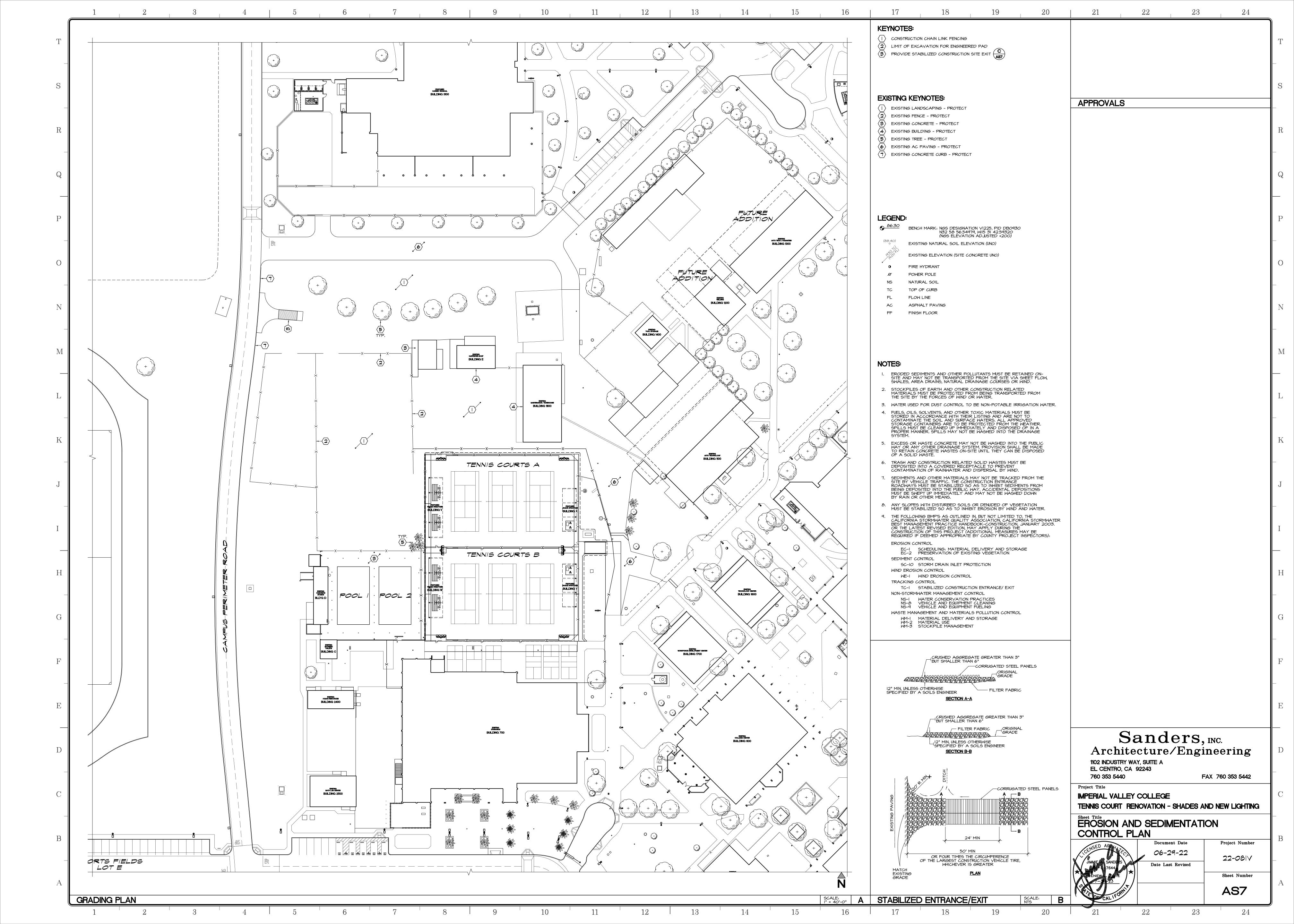


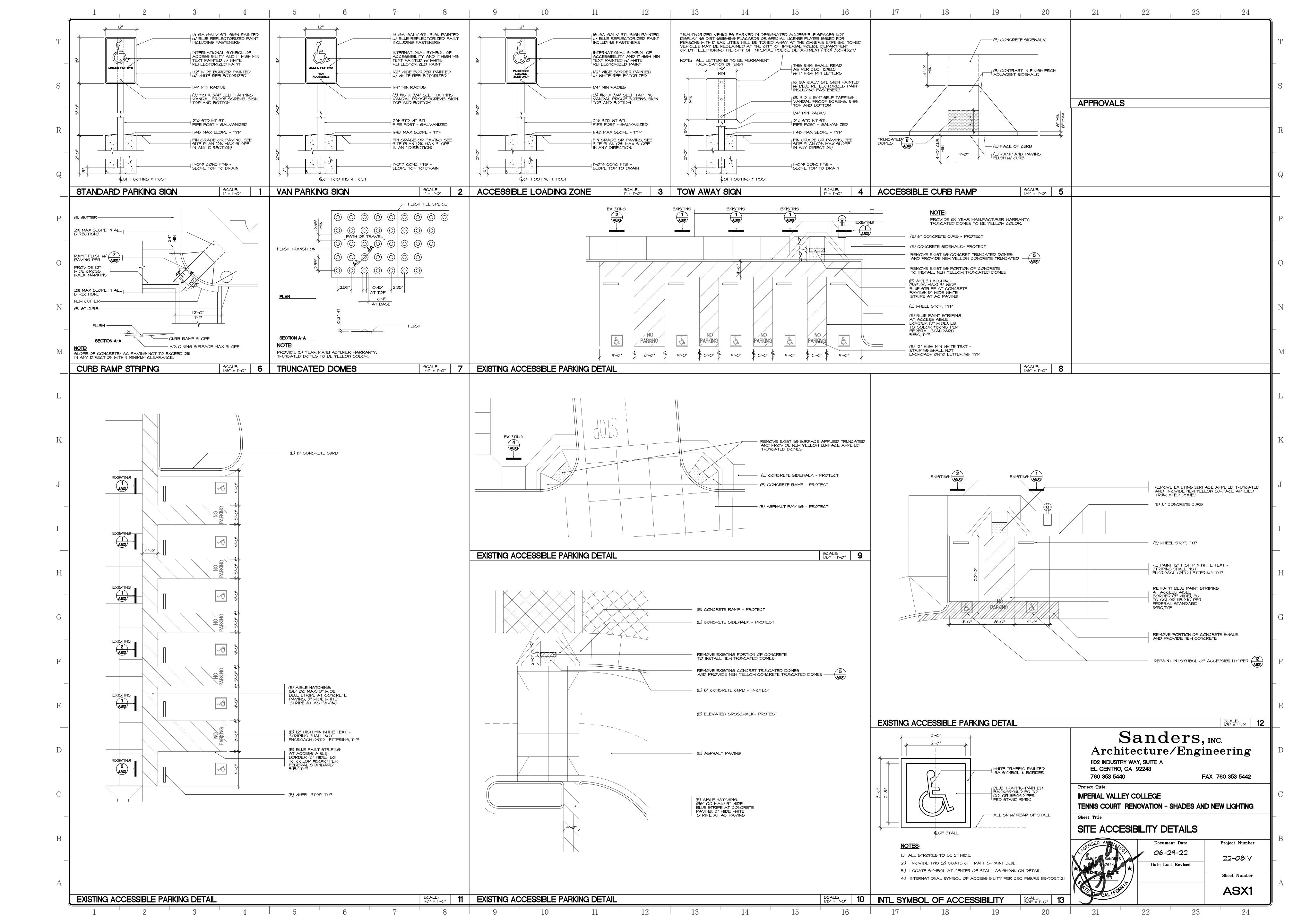


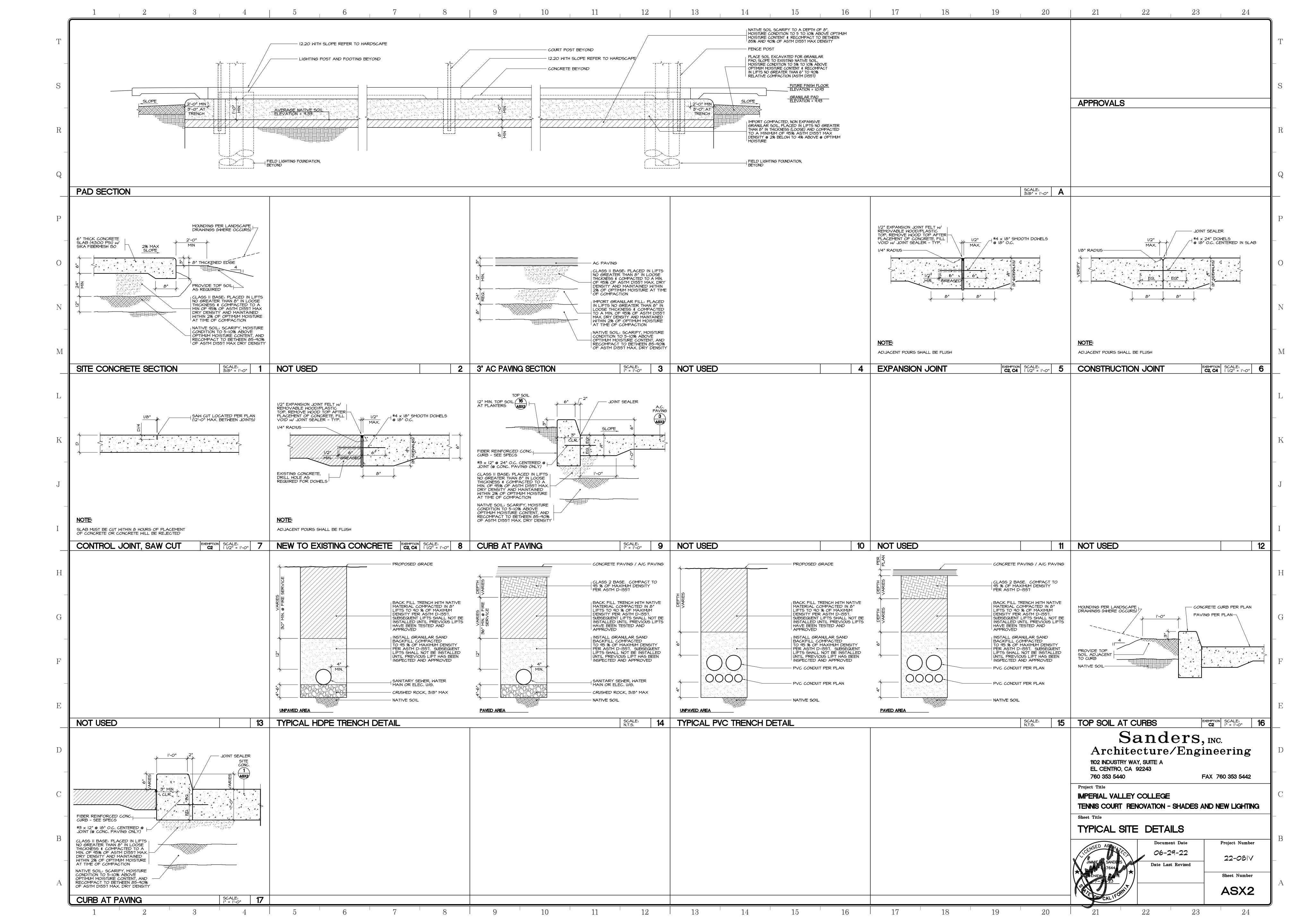


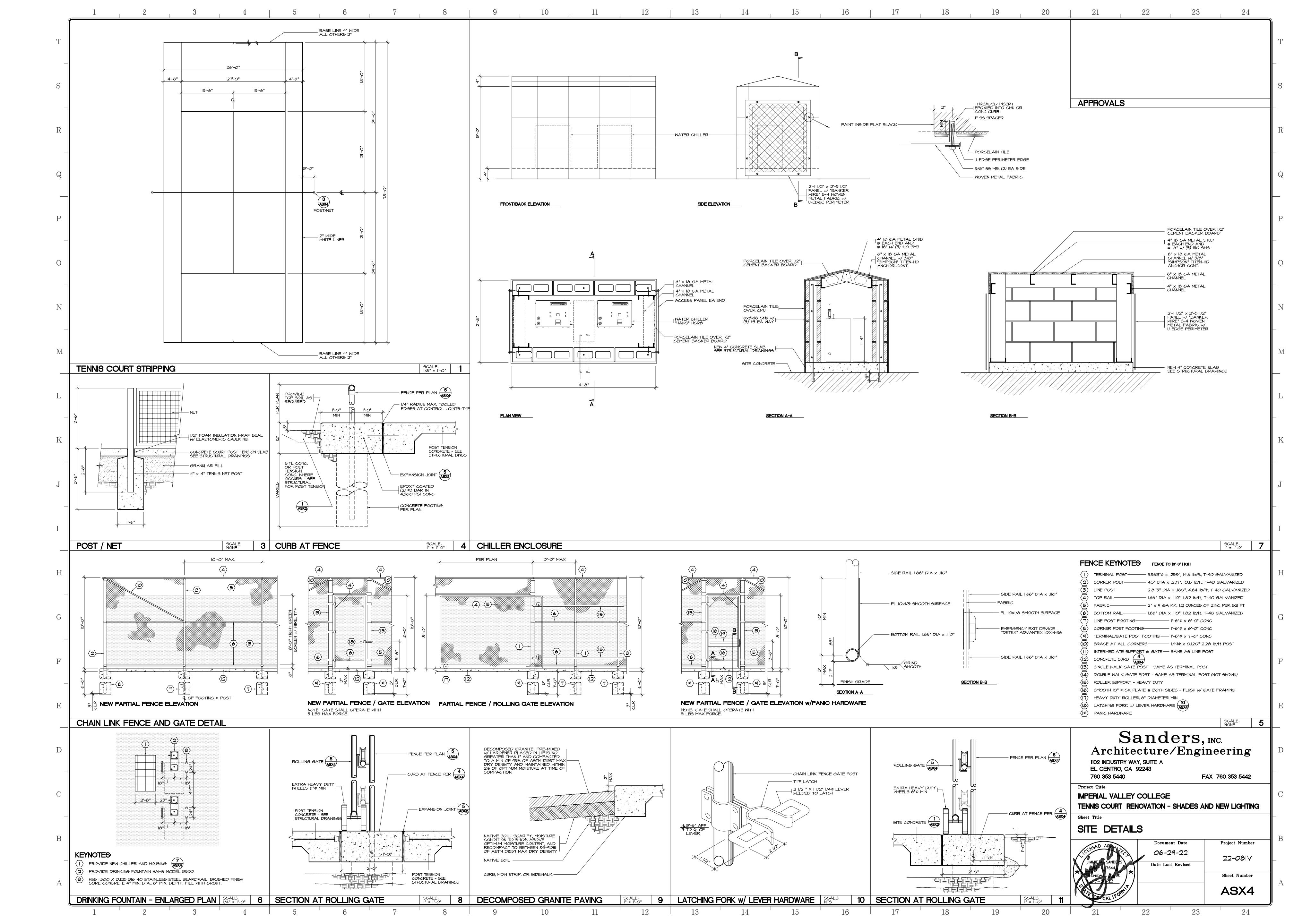


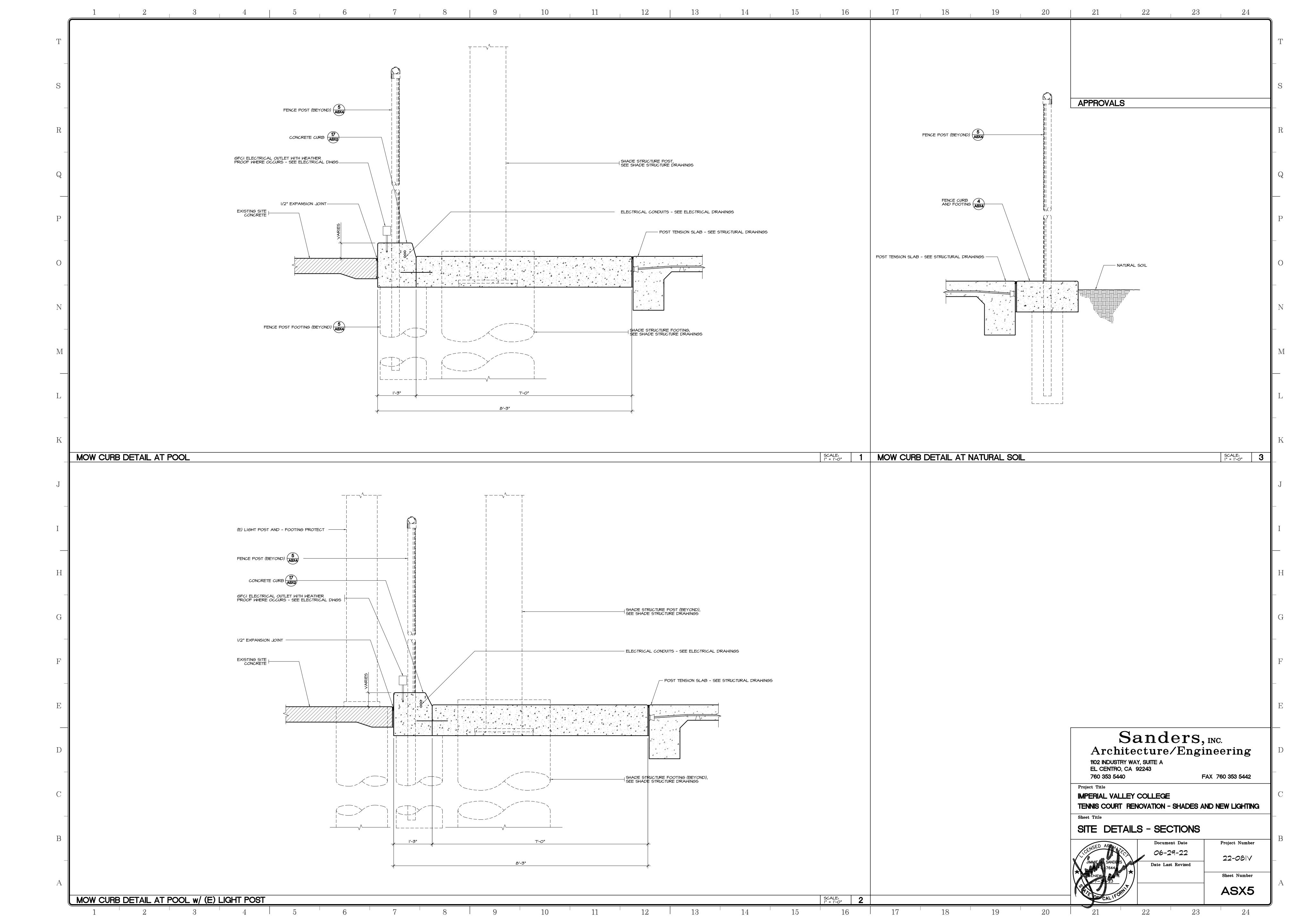


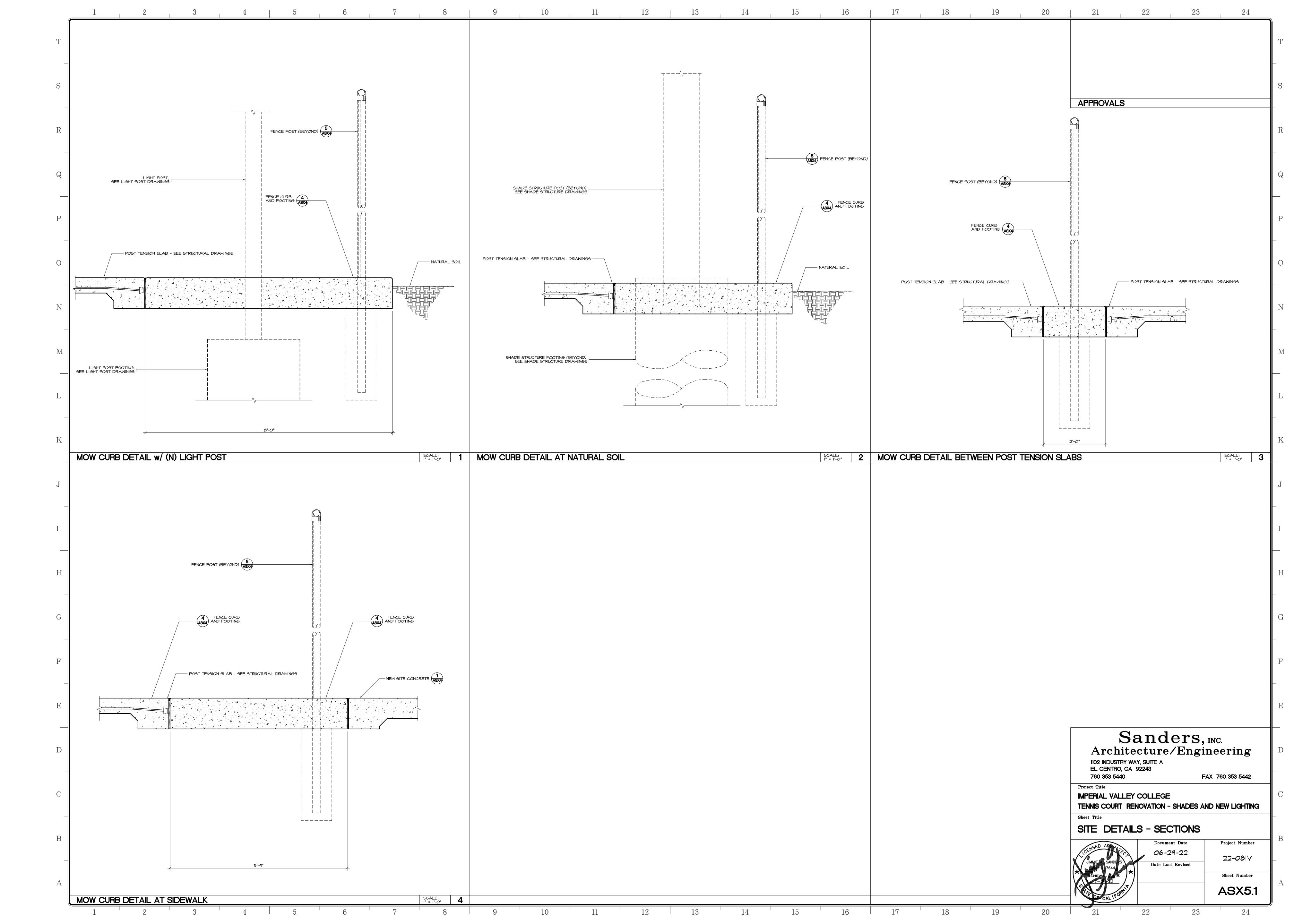


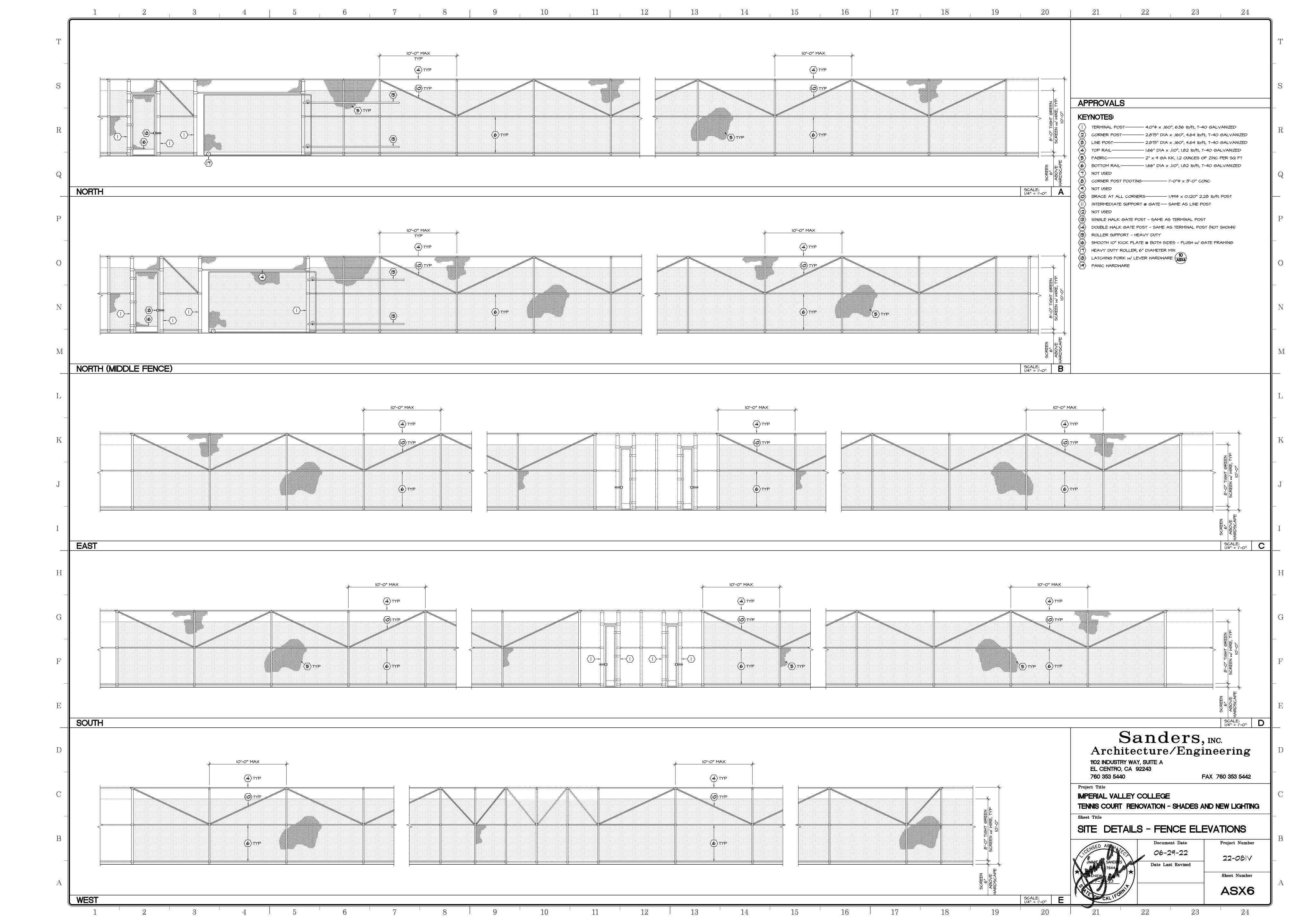


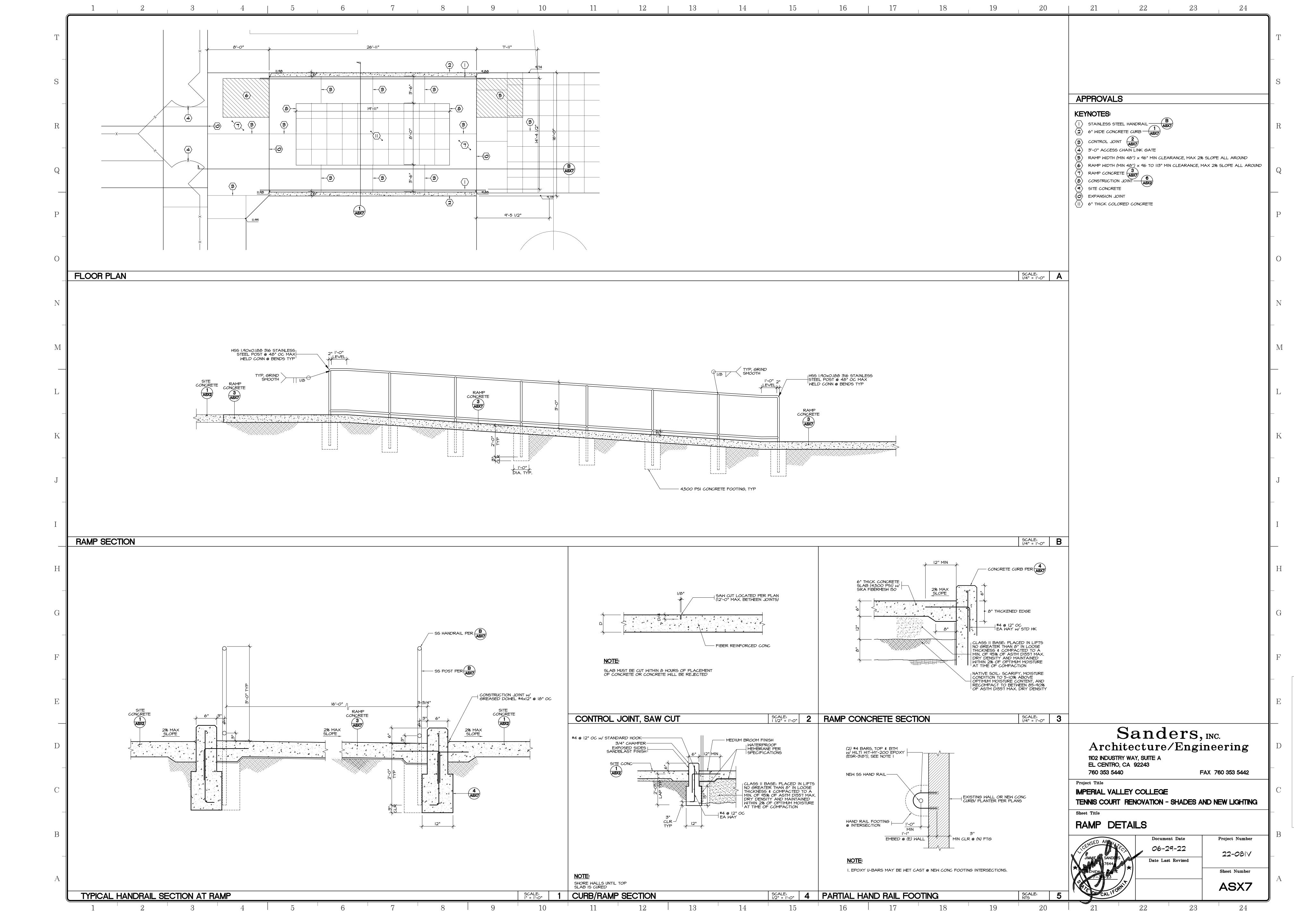


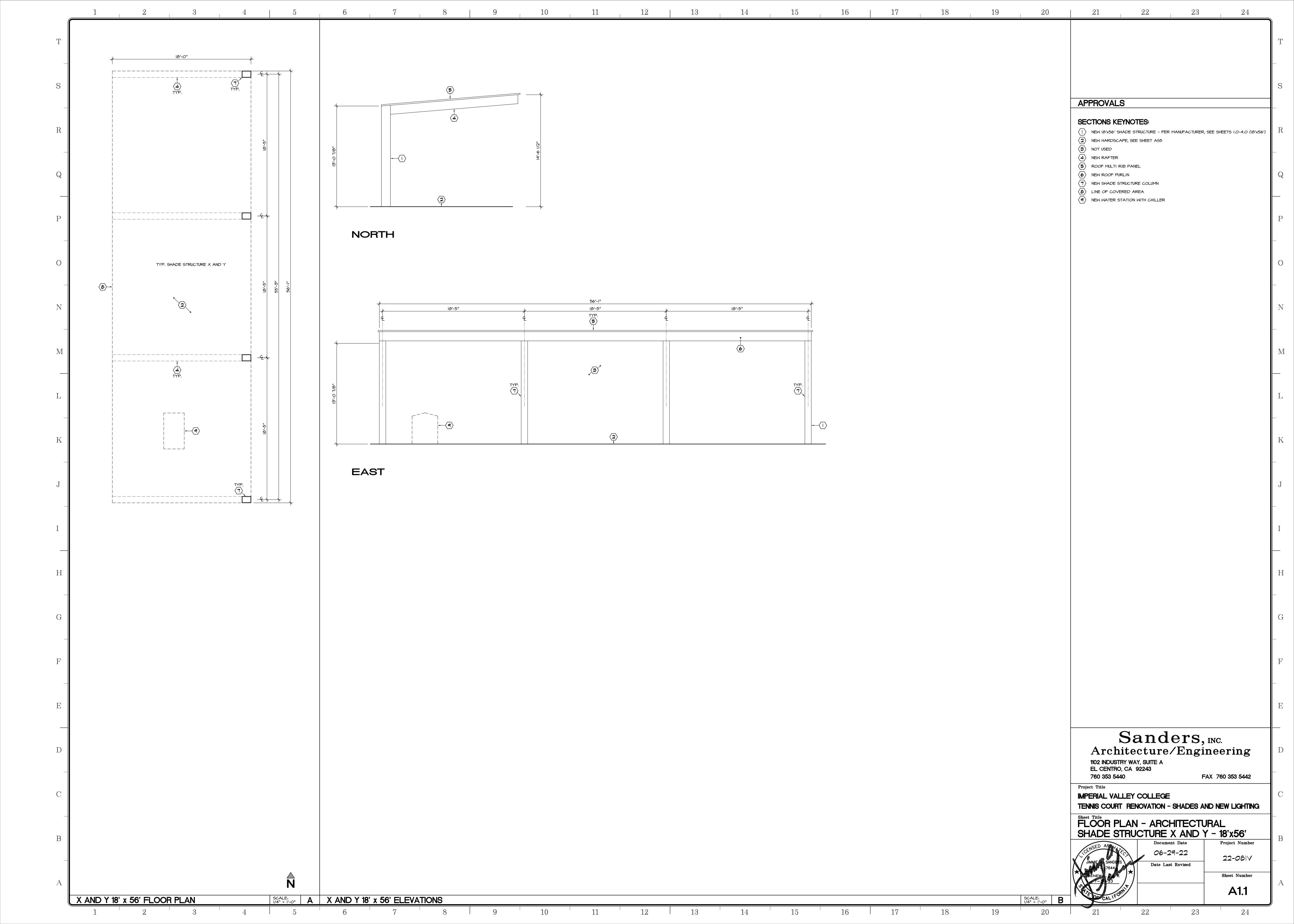


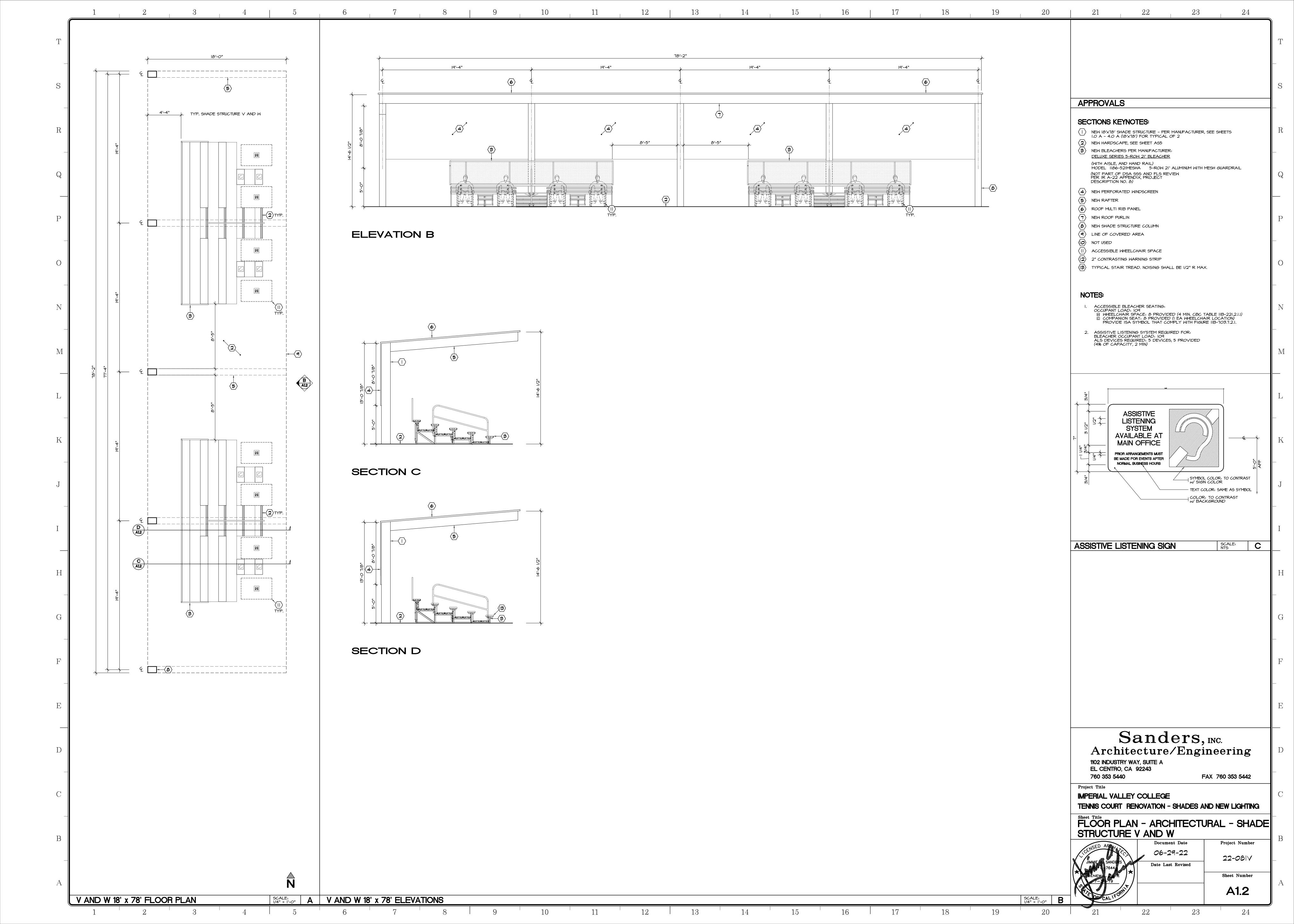


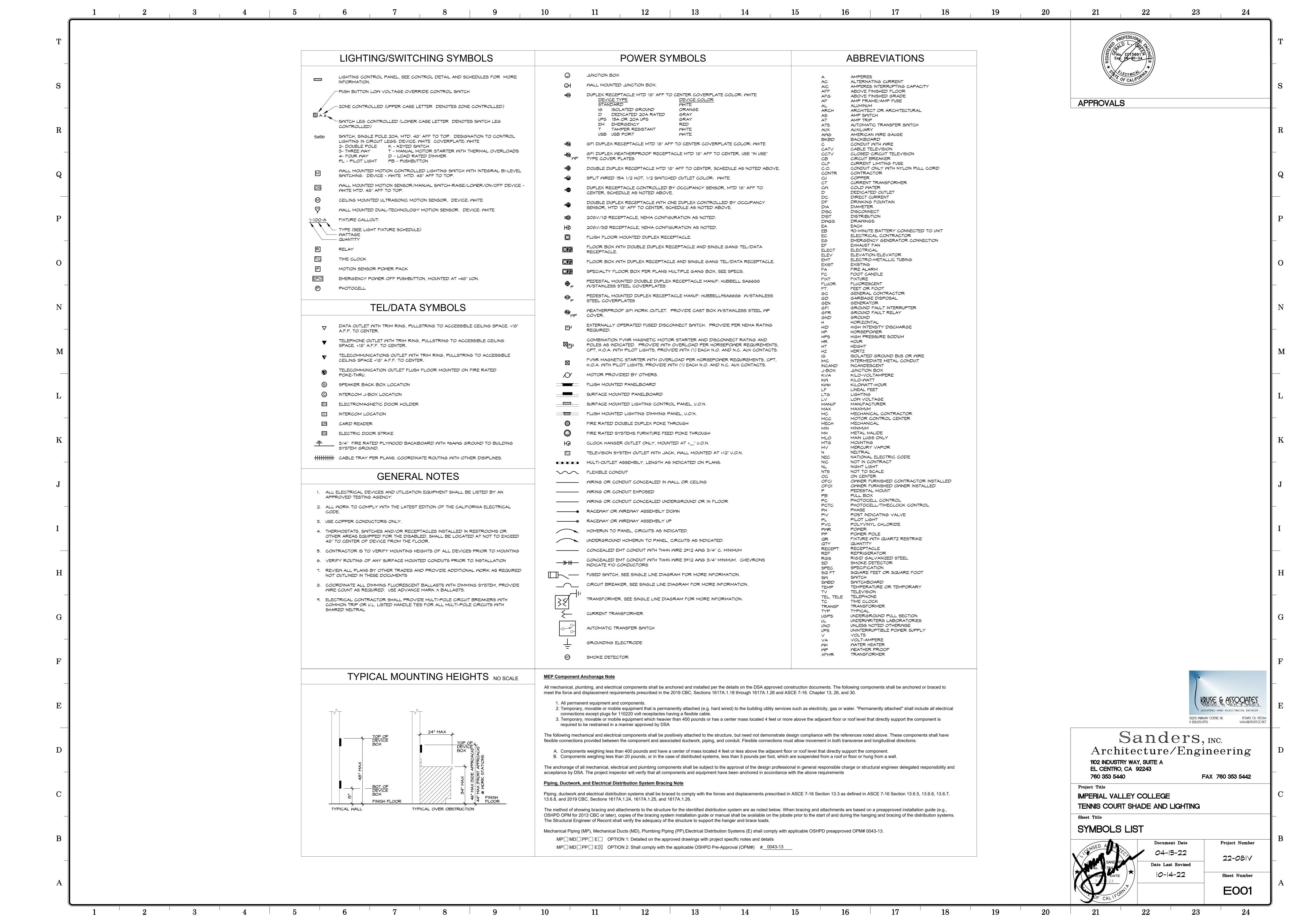


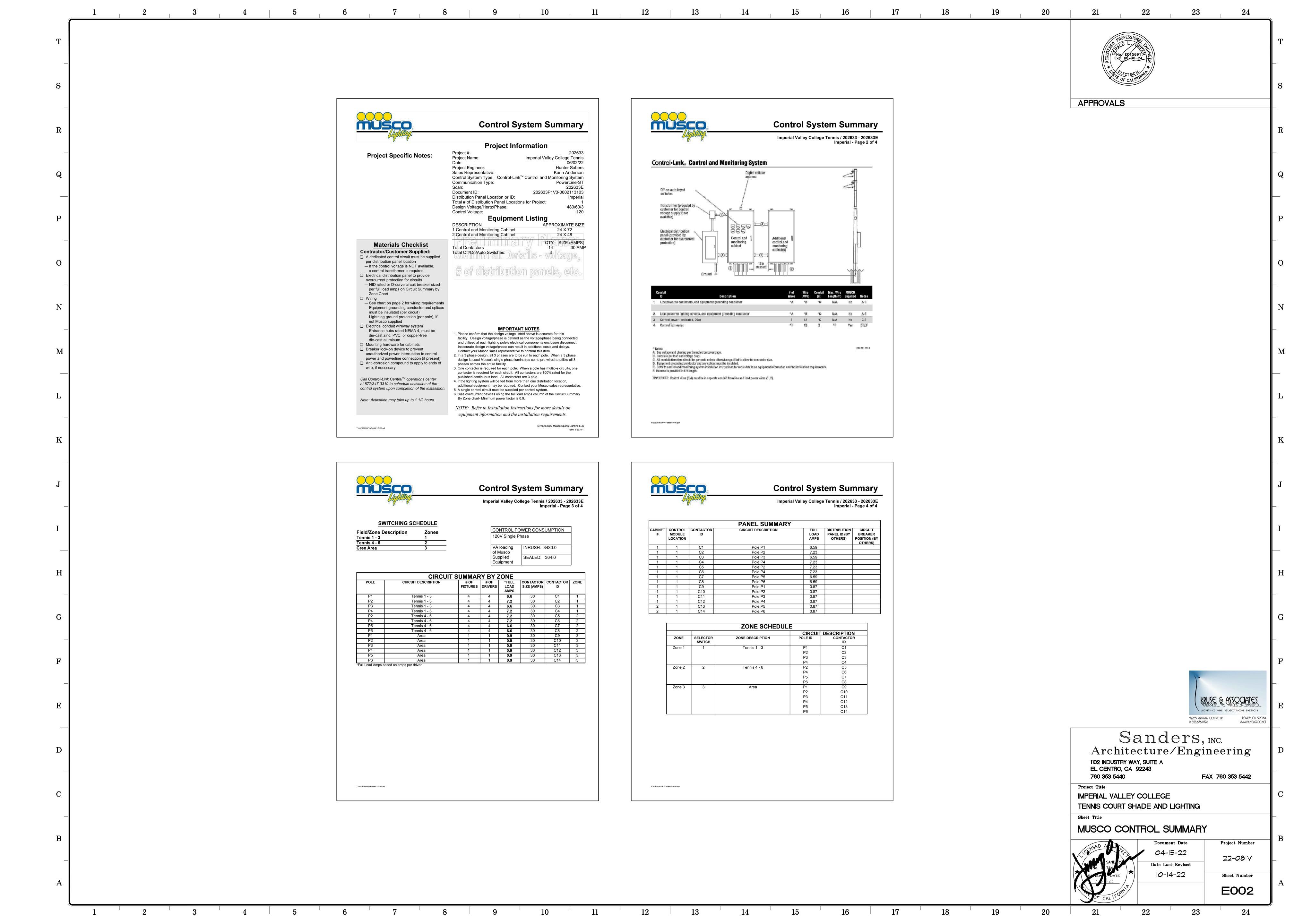


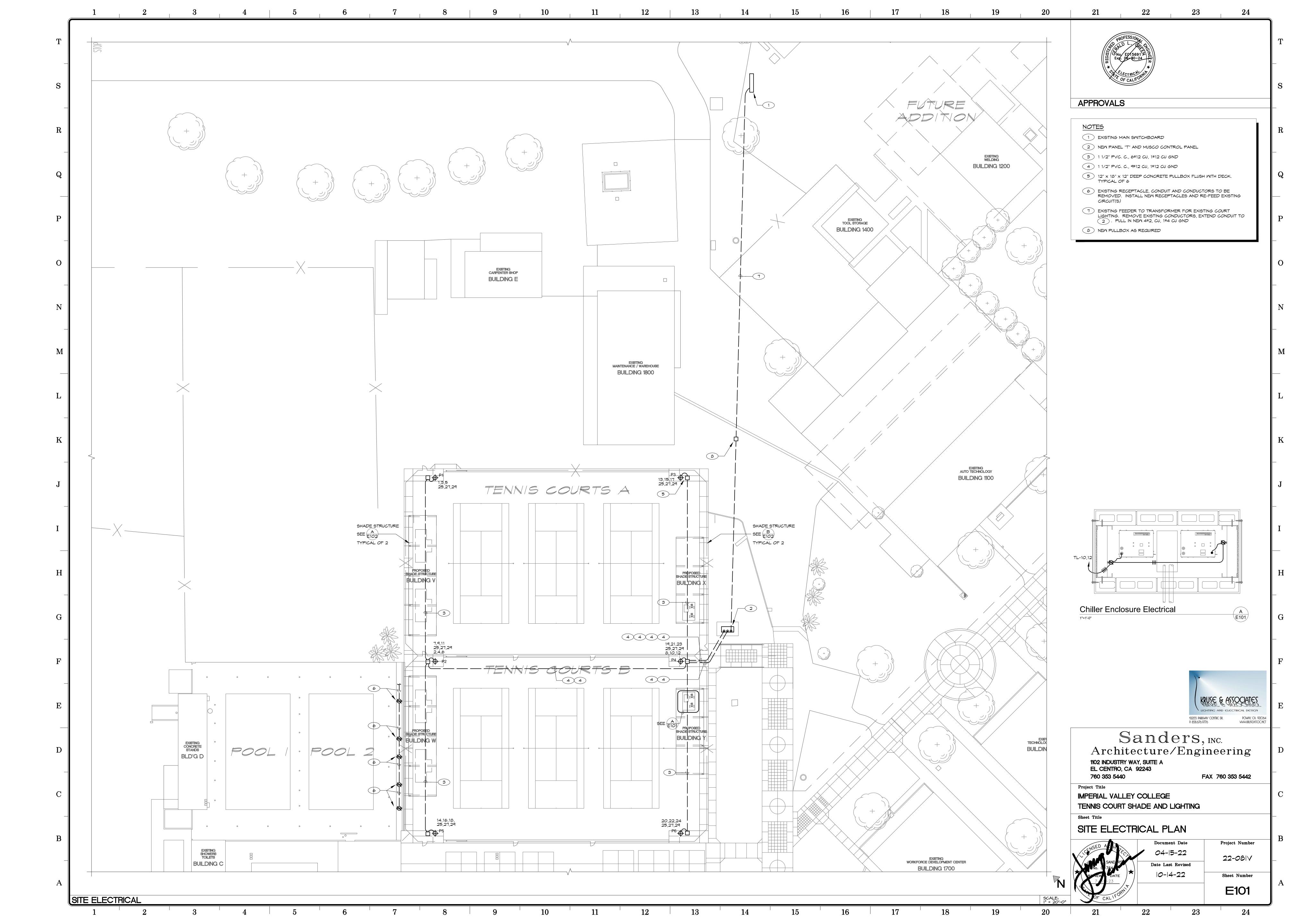


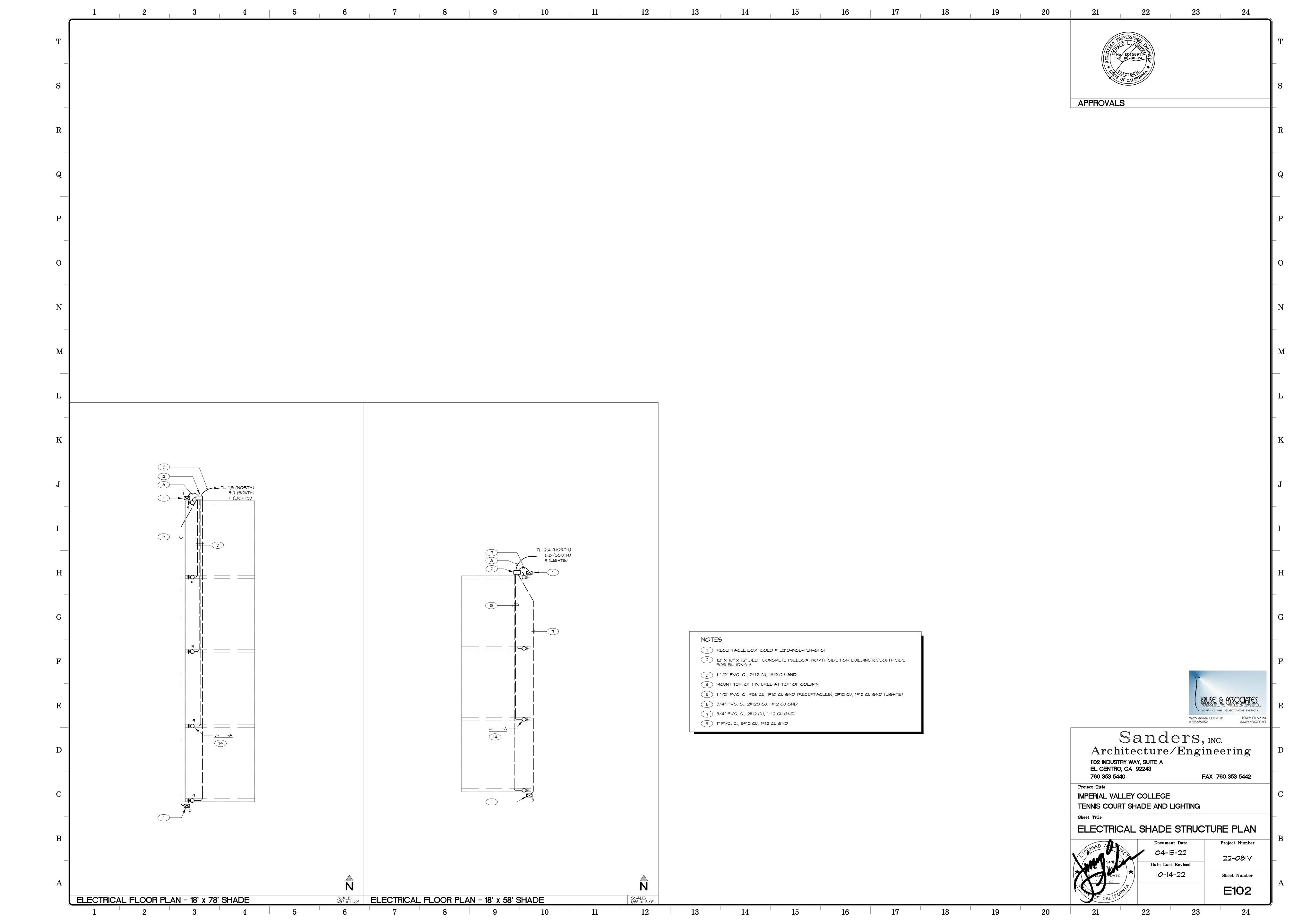


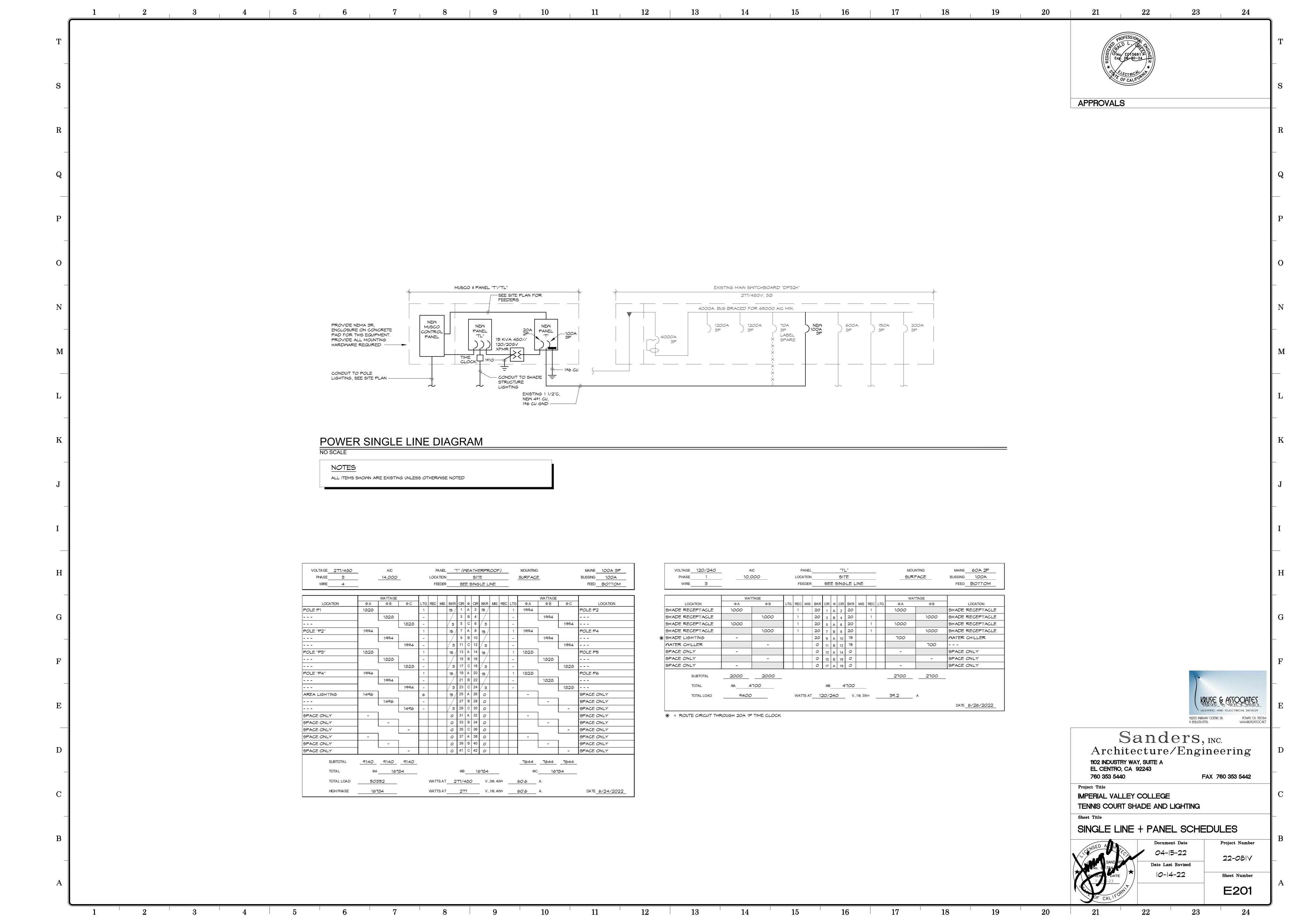












GENERAL

- 1. SPECIFIC NOTES & DETAILS ON THE SHADE STRUCTURE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES & TYPICAL DETAILS
- WHERE NO DETAILS ARE SHOWN OR NOTED IN ANY PART OF THE WORK THE DETAILS FOR OTHER SIMILAR WORK SHALL APPLY.
- 3. DETAILS IDENTIFIED AS TYPICAL, SHALL APPLY IN ESTIMATING AND CONSTRUCTION TO EVERY LIKE CONDITION WHETHER OR NOT THE 4. THE STRUCTURAL DRAWINGS SHALL NOT BE SCALED. COORDINATE
- DIMENSIONS WITH ARCHITECTURAL DRAWINGS. 5. COORDINATE ELEVATIONS, SLOPES AND DRAINAGE REQUIREMENTS
- WITH THE ARCHITECTURAL DRAWINGS. 6. STANDARDS REFERENCED ON THE STRUCTURAL DRAWINGS REFER
- 7. THE RESPONSIBILITY FOR THE REVIEW AND COORDINATION OF DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF RELATED CONSTRUCTION SHALL BEAR ON THE CONTRACTOR. DISCREPANCIES THAT EXIST SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER IN A TIMELY MANNER, PRIOR TO START OF RELATED CONSTRUCTION.

TO THE EDITION APPLICABLE UNDER THE CURRENT BUILDING CODE.

- WORK PERFORMED IN CONFLICT WITH THE STRUCTURAL DRAWINGS OR APPLICABLE BUILDING CODE REQUIREMENTS SHALL BE CORRECTED AT THE EXPENSE OF THE CONTRACTOR.
- DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS SHALL BE VERIFIED BEFORE STARTING RELATED WORK AND THE ENGINEER

NOTIFIED OF DISCREPANCIES IN A TIMELY MANNER.

- 10. SITE CONDITIONS THAT ARE NOT REFLECTED ON THE STRUCTURAL DRAWINGS OR THAT DEVIATE FROM THE MAXIMUM OR MINIMUM DIMENSIONS INDICATED SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER IN A TIMELY MANNER. SUCH CONDITIONS MAY INCLUDE CONFLICT IN GRADES, ADVERSE SOIL CONDITIONS, GROUND WATER PRESENT, DEEPENED FOOTINGS, UNCOVERED AND UNEXPECTED UTILITY LINES, ETC.
- 11. MATERIALS AND WORKMANSHIP SHALL CONFORM TO REQUIREMENTS OF APPLICABLE REGULATIONS AND THE BUILDING CODE AS AMENDED AND ADOPTED BY THE BUILDING OFFICIAL.

FOUNDATION

THE STRUCTURE WILL BE LOCATED ENTIRELY ON UNDISTURBED NATIVE SOIL. IF THE BUILDING INSPECTOR SUSPECTS FILL, EXPANSIVE SOILS OR ANY GEOLOGIC INSTABILITY BASED UPON OBSERVATION OF THE FOUNDATION EXCAVATION, A SOILS OR GEOLOGICAL REPORT, AND RESUBMITTAL OF PLANS TO PLAN CHECK

TO VERIFY THAT THE REPORT RECOMMENDATIONS HAVE BEEN

INCORPORATED, MAY BE REQUIRED.

- TIMOTHY K. JAQUESS, SE LICENSED ENGINEER
- THE MAXIMUM ALLOWABLE SOIL BEARING PRESSURE SHALL BE 1500 psf. THE RESULTING ALLOWABLE BEARING VALUE MAY BE INCREASED BY 1/3 FOR WIND AND SEISMIC LOAD CASES.
- FOOTING DEPTHS INDICATED ON THE STRUCTURAL DRAWINGS ARE FOR BIDDING PURPOSES ONLY AND ARE ASSUMED TO BE IN SUITABLE BEARING MATERIALS.

FOOTING ELEVATIONS SHALL BE LOCATED SUCH THAT THEIR

- BEARING IS A MINIMUM HORIZONTAL DISTANCE OF 10 FEET FROM THE DAYLIGHT OF AN ADJACENT SLOPE.
- ANCHOR BOLTS, DOWELS AND HOLD-DOWN ANCHORS SHALL BE TIED IN PLACE PRIOR TO FOUNDATION INSPECTION.

SUBMITTALS

- 1. THE CONTRACTOR SHALL MAKE SUBMITTALS PRIOR TO FABRICATION AS REQUIRED BY THE WRITTEN SPECIFICATIONS AND SHALL INCLUDE AS A MINIMUM THE FOLLOWING SUBMITTALS:
- CONCRETE MIX DESIGNS
- REINFORCING STEEL DRAWINGS STRUCTURAL STEEL DRAWINGS
- METAL DECK DRAWINGS WELDING PROCEDURES (SHOP AND FIELD WELDING).
- 2 THE FOLLOWING SHOP DRAWINGS ARE NOT REQUIRED FOR SUBMISSION FOR STRUCTURAL REVIEW:
- SHORING AND BRACING UNSPLICED REBAR AT SLAB-ON-GRADE AND FOOTINGS
- FORMWORK D. STRUCTURAL STEEL MILL REPORTS
- STEEL REINFORCING LISTS AND QUANTITIES AND LENGTHS OF ALL MATERIALS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO ASSURE COMPLIANCE WITH THE PLANS. ENGINEER WILL NOT
- SHALL BE STAMPED AND SIGNED BY THE CONTRACTOR INDICATING THE CONTRACTORS PRIOR REVIEW AND THAT THE SUBMITTAL IS IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

SHOP DRAWINGS SUBMITTED TO THE ENGINEER FOR REVIEW

AS AN ALTERNATE TO MULTIPLE PAPER COPIES, SHOP DRAWINGS MAY BE SUBMITTED IN ELECTRONIC (PDF) FORMAT. WHERE SUBMITTED ELECTRONICALLY, SHOP DRAWINGS WILL BE PROCESSED AND RETURNED ELECTRONICALLY.

REINFORCING STEEL

- DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS SHALL FOLLOW ACI 315, "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT."
- 2. REINFORCING BARS SHALL CONFORM TO ASTM A 615, GRADE 60,
- 3. WELDED REINFORCING BARS SHALL CONFORM TO ASTM A 706, GRADE 60. BARS TO BE WELDED CONFORMING TO ASTM A 615, GRADE 60 MAY BE SUBSTITUTED PROVIDED THAT THE WELDING OF BARS COMPLIES WITH AWS D1.4 AND THE MINIMUM SPECIFICATIONS FOR WELDING OF REINFORCING STEEL INCLUDED HEREIN.
- 4. REINFORCING BAR LAP SPLICES SHALL BE CLASS B. (18" MIN.) FOR CONCRETE, U.O.N.
- 5. DETAILS OF REINFORCEMENT SHALL COMPLY WITH ACI 318, CHAPTER 25.

CONC. CAST AGAINST EARTH

- 6. REINFORCING BARS FOR CONCRETE SHALL BE PROVIDED WITH THE FOLLOWING MINIMUM COVER:
 - FORMED CONC. EXPOSED TO EARTH/WEATHER: #5 OR SMALLER #6 OR LARGER SLABS (#11 AND SMALLER) BEAMS & GIRDERS
- 7. #3 SPACER TIES SHALL BE INSTALLED AT 30" ON CENTER IN ALL BEAMS AND FOOTINGS TO SECURE REINFORCING BARS IN PLACE,
- 8. AT THE CONTRACTOR'S OPTION, INTERNALLY-THREADED LENTON A2 COUPLERS (IAPMO UES-0129) MAY BE USED IN LIEU OF LAP SPLICES.

MECHANICAL BAR SPLICES MAY BE TYPE 1 OR TYPE 2 AS DEFINED IN

REINFORCED CONCRETE

19A OF THE BUILDING CODE AND TO THE PROVISIONS OF ACI

- CONCRETE CONSTRUCTION SHALL CONFORM WITH CHAPTER
- 318, LATEST EDITION. READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN
- ACCORDANCE WITH ASTM C 94. CEMENT SHALL CONFORM TO ASTM C 150 TYPE I OR II. LOW
- ALKALI, OR TYPE V FOR SULFATE EXPOSURE CLASS S2 AND S3. AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL
- CONFORM TO ASTM C 33.
- 5. NORMAL WEIGHT CONCRETE SHALL HAVE A MAXIMUM DRY DENSITY OF 150 PCF. 6. CONCRETE MIXES SHALL BE DESIGNED BY A QUALIFIED TESTING LABORATORY AND APPROVED BY THE ENGINEER. MIX
- DESIGN METHODS (TEST HISTORY OR TRIAL BATCH METHOD) IN PROPORTION CONCRETE.
- ACCORDANCE WITH ACI 318, SECTION 26.4 SHALL BE USED TO
- MINIMUM CONCRETE COMPRESSIVE STRENGTHS AT 28 DAYS, MAXIMUM SLUMPS, AND MAXIMUM WATER/CEMENT RATIOS SHALL BE AS FOLLOWS:

DESCRIPTION	MIN 28 DAY f'c	<u>SLUMP</u>	MAX RA
FOOTINGS	3000 PSI	4"+/-1'	0.5
ALL OTHER CONCRETE	3000 PSI	4"+/-1"	0.5

- 8. ADMIXTURES SHALL BE APPROVED IN ADVANCE.
- 9. SLUMPS INDICATED ARE PRIOR TO PLASTICIZER ADDITIVES.
- 10. CONCRETE ADMIXTURES CONTAINING CHLORIDE OR CHLORIDE SALTS SHALL NOT BE USED EXCEPT WHERE APPROVED IN WRITING BY THE ENGINEER.
- 11. FLYASH SHALL BE LIMITED TO NO MORE THAN 20% OF THE TOTAL WEIGHT OF CEMENTITIOUS MATERIALS IN THE CONCRETE, U.O.N.
- 12. WATER MAY BE ADDED ON SITE TO OBTAIN SPECIFIED SLUMPS PROVIDED THAT IT IS ADDED WITHIN ONE HOUR OF BATCHING AND SITE-ADDED WATER IS SPECIFIED ON THE BATCH REPORT. SITE-ADDED WATER SHALL NOT COMPROMISE THE STRENGTH OR SLUMP OF THE CONCRETE.
- 13. CONCRETE SHALL NOT BE PLACED BEYOND 1-1/2 HOURS FOLLOWING BATCHING.
- 14. CONDUIT OR PIPES LARGER THAN 4" NOMINAL DIAMETER SHALL NOT BE PLACED IN CONCRETE. SLEEVES FOR OPENINGS IN CONCRETE SHALL BE INSTALLED BEFORE PLACING REINFORCING WHICH MAY CONFLICT SHALL NOT BE CUT UNLESS APPROVED IN WRITING BY THE ENGINEER.
- 15. STEEL COLUMNS, BEAMS PURLINS & PANELS MAY BE ERECTED 24 HOURS AFTER FOUNDATION CONCRETE PLACEMENT OR AFTER CONCRETE REACHES A COMPRESSIVE STRESS OF 1000 PSI, WHICHEVER COMES FIRST.

STRUCTURAL STEEL

- 1. STRUCTURAL STEEL WORK SHALL BE PERFORMED IN ACCORDANCE WITH CHAPTER 22 OF THE BUILDING CODE, AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND
- 2. STRUCTURAL STEEL STRENGTHS AND GRADES SHALL BE AS FOLLOWS, U.O.N.:

<u>DESCRIPTION</u> ANGLES, CHANNELS	<u>YIELD</u> <u>Fy,ksi</u>	<u>ASTM</u> GRADE
& OTHER SHAPES	36ksi	A36
HSS SECTIONS	46ksi	A500, GR B
CONNECTION PLATES & MISC., U.O.N.	50ksi	A572 GR 50

- 3. ANCHOR RODS SHALL CONFORM TO ASTM F 1554, GRADE 105, UNLESS OTHERWISE NOTED. NUTS FOR ANCHOR RODS SHALL CONFORM TO ASTM A 563, GRADE C HEX (HEAVY HEX WHERE ANCHOR ROD DIAMETER IS GREATER THAN 1 1/2").
- 4. MAIN MEMBER SHALL HAVE HIGH STRENGTH BOLTS CONFORMING TO AISC SPECS FOR ASTM A 325N BOLTS, U.O.N. OTHER BOLTS SHALL CONFORM TO ASTM A 307. NUTS FOR HIGH STRENGTH BOLTS SHALL BE HEAVY HEX GRADE C CONFORMING TO ASTM
- 5. EXTERIOR STRUCTURAL STEEL PERMANENTLY EXPOSED TO THE WEATHER SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION.
- ZINC COATING SHALL CONFORM TO ASTM A123 (G60, U.O.N.). 6. TIGHTEN ASTM A 325N BOLTS TO "SNUG-TIGHT" CONDITION PER AISC SPECIFICATION FOR STRUCTURAL JOINTS. TEST ASTM A
- 325SC BOLTS WITH A CALIBRATED WRENCH UNLESS LOAD INDICATOR BOLTS ARE USED.

SLOPED SURFACES.

WELDING DONE AFTER GALVANIZING SHALL BE PROTECTED ACCORDING TO ASTM A780.

PROVIDE BEVELED WASHERS PER ANSI B18.23.1 AS REQUIRED ON

9 FAYING SURFACES OF HOT-DIPPED GALVANIZED MEMBERS AND CONNECTORS SHALL BE ROUGHENED WITH A HAND WIRE-BRUSH PRIOR TO ERECTION. ROUGHENING PROCESS SHALL VISIBLY ALTER THE GALVANIZED SURFACE WITHOUT DISRUPTING THE CONTINUITY OF GALVANIZATION. POWER WIRE-BRUSHING NOT ALLOWED.

WELDING

- 1. WELDING SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF THE AMERICAN WELDING SOCIETY (AWS D1.1).
- 2. WELDING OF METAL DECK AND LIGHT GAUGE METAL FRAMING SHALL BE DONE BY CERTIFIED LIGHT GAUGE WELDERS IN ACCORDANCE WITH AWS SPECIFICATIONS FOR WELDING SHEET STEEL IN STRUCTURES, AWS D1.3.
- 3. WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS.
- 4. WELDING ELECTRODE FOR THE SHIELDED METAL-ARC WELDING (S.M.A.W.) PROCESS AND WELDING ELECTRODE SHALL CONFORM TO AWS A5.1 "SPECIFICATION FOR CARBON STEEL ELECTRODES FOR SHIELDED METAL ARC WELDING."
- ELECTRODES FILLER MATERIAL SHALL BE A MINIMUM OF E70XX U.O.N., EXCEPT E60XX MAY BE USED FOR WELDING OF METAL DECK AND LIGHT GAUGE FRAMING.
- 6. WELDS SHALL HAVE WELD CONTROLLED SEQUENCE AND TECHNIQUE IN ORDER TO MINIMIZE SHRINKAGE STRESSES AND DISTORTION.

METAL DECK

METAL DECK SHALL BE OF THE TYPE AND GAUGE AS INDICATED ON THE DRAWINGS AND SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND PUBLISHED ICC

METAL DECK AND ACCESSORIES SHALL BE FORMED FROM STEEL

- SHEETS CONFORMING TO ASTM A 653, WITH COATING DESIGNATION G60 [G90].
- 3. DECK SHALL BE CONTINUOUS OVER THREE SPANS WHEREVER POSSIBLE. SHORE DECK IF RECOMMENDED BY THE MANUFACTURER. MINIMUM BEARING AT ENDS IS 2".

4. WHERE 3/4" DIAMETER SHEAR STUDS ARE TO BE WELDED TO

5. CONTRACTOR SHALL CUT DECK PER STRUCTURAL DETAILS AT ALL OPENINGS, COLUMNS, AND REQUIRED PENETRATIONS AND SHALL SUPPLY NECESSARY ACCESSORY ITEMS SUCH AS CLOSURES, CLIPS, ETC.

SUPPORTS, 18 GAUGE (OR THICKER) DECKING SHALL NOT BE

SHEET METAL SCREWS

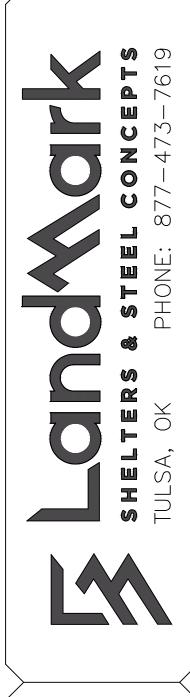
- 2. SCREWS TO CONFORM TO THE REQUIREMENTS OF ASTM C1513.
- 3. UNLESS OTHERWISE NOTED, SCREWS SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
- SELF-TAPPING D. HEX-HEAD
- DURING INSTALLATION OF SCREWS, THE SCREW SHALL PENETRATE BOTH PLIES OF MATERIAL TO BE ATTACHED, WITH A MINIMUM OF 3 THREADS OF SCREW TIP EXPOSED.
- 6. SCREWS SHALL MEET THE FOLLOWING MINIMUM STRUCTURAL REQUIREMENTS:

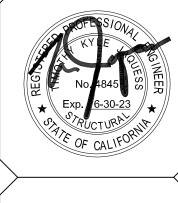
ALLOWABLE SHEAR	ALLOWABLE PULL-OUT
200#	99#
308#	132#
430#	165#
601#	207#
724#	289#
	SHEAR 200# 308# 430# 601#

- SCREWS SHALL BE ITW BUILDEX "TEKS" PER ICC ESR-1976 AND SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS.
- ZINC-PLATED HARDENED STEEL
- 5. SCREWS SHALL BE TIGHTENED IN SHUCH A MANNER THAT THE WASHER-FACE OF SCREW HEAD IS FLUSH WITH ADJACENT PLY, AND PLIES OF MATERIAL TO BE FASTENED ARE DRAWN TOGETHER SNUGLY.

TEGORIEMENTO.		
THINNER OF PLIES TO BE JOINED	ALLOWABLE <u>SHEAR</u>	ALLOWABLE PULL-OUT
20 GA.	200#	99#
18 GA.	308#	132#
16 GA.	430#	165#
14 GA.	601#	207#
12 GA.	724#	289#

DESCRIPTION DEAD & LIVE LOADS ROOF LIVE LOAD ROOF PANEL DEAD LOAD ROOF PANEL DEAD LOAD 1.1 PSF WIND DESIGN BASIC WIND SPEED (3 SECOND GUST), Vult RISK CATEGORY II EXPOSUBE CATEGORY FACTORS: Kz, KzT (1 MINIMUM) Kd Q.85 / 1.0 / 0.85 A 0.00256 Kz KzT Kd V2 12.74 PSF CLEAR WIND FLOW GIND (X] YES OBSTRUCTED WIND FLOW SEISMIC DESIGN LATERAL FORCE-RESISTING SYSTEM ANY YSIS PROCEDURE SEISMIC DESIGN ANY YSIS PROCEDURE SEISMIC DESIGN CATEGORY (5DC) E SEISMIC BESIGN CATEGORY (5DC) FE SEISMIC RESPONSE COEFFICIENT, CS DISSIMIC STREED CATEGORY (5DC) SEISMIC RESPONSE COEFFICIENT, CS ASSESSMIC RESPONSE COEFFICIENT, CS ASSESSMIC RESPONSE COEFFICIENT, CS ANY SEISMIC ATTOO ACTOR, R OVERSTRENGTH FACTOR, R OVERSTRENGTH FACTOR, R 1.25 SHORT-PERIOD SITE COEFFICIENT, F9 LOPSIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, SIG-UNITE COEFFICIENT, F9 LONG PERIOD SITE COEFFICIENT, F9 LONG PERIOD S	DESIGN CRITERIA	
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ROOF PANEL DEAD LOAD **NIND DESIGN** **WIND DESIGN** **BASIC WIND SPEED (3 SECOND GUST), Vult** **RISK CATEGORY** **EXPOSURE CATEGORY** **EXPOSURE CATEGORY** **GENOME CATEGORY** **FACTORS: Kz, Kzf (1 MINIMUM) Kd** **GENOME CATEGORY** **SEISMIC DESIGN** **LATERAL FORCE-RESISTING SYSTEM** **SEISMIC DESIGN** **LATERAL FORCE-RESISTING SYSTEM** **SEISMIC DESIGN CATEGORY** **SEISMIC DESIGN CATEGORY** **SEISMIC SITE CLASS** **D **SEISMIC IMPORTANCE FACTOR, Ie** **D **D **SEISMIC IMPORTANCE FACTOR, Ie** **D **D **SEISMIC RESPONSE COEFFICIENT, C9** **SEISMIC RESPONSE COEFFICIENT, C9** **D **SEISMIC MODIFICATION FACTOR, R** **OVERSTRENGTH FACTOR, Q** **D **D **D **SEISMIC STEC CASS** **D **OVERSTRENGTH FACTOR, Q** **D **D **D **SEISMIC STEC CASS** **D **OVERSTRENGTH FACTOR, Q** **D **D **D **D **SEISMIC STEC CASS** **D **OVERSTRENGTH FACTOR, Q** **D **D **D **D **D **D **D	DEAD & LIVE LOADS	
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Sds-USED TO DETERMINE Cs (WITH CAP PER CBC 1616.10.00) MAPPED SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, S1 LONG PERIOD, S1 LONG PERIOD SITE COEFFICIENT, FV 1.70 HORIZONTAL OR VERTICAL IRREGULARITIES TYPE (S) ALLOWABLE SHAFT RESISTANCE FOR FOUNDATIONS CBC TABLE 125 PSF DOWN, 100 PSF UP 1500 PSF VERTICAL, 100 PSF/FT LATERAL FLOOD DESIGN - DESIGN ASSUMED TO NOT BE IN FLOOD HAZARD AREA IF PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ARCHITECTURAL REQUIREMENTS DESCRIPTION DESIGN VALUES TYPE OF CONSTRUCTION II-B OCCUPANCY CLASSIFICATION A-3	SHORT-PERIOD SITE COEFFICIENT, Fa	1.2
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OCCUPANCY CLASSIFICATION A-3	DESCRIPTION	DESIGN VALUES
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	NUMBER OF STORIES	1







IVC - IMPERIAL VALLEY

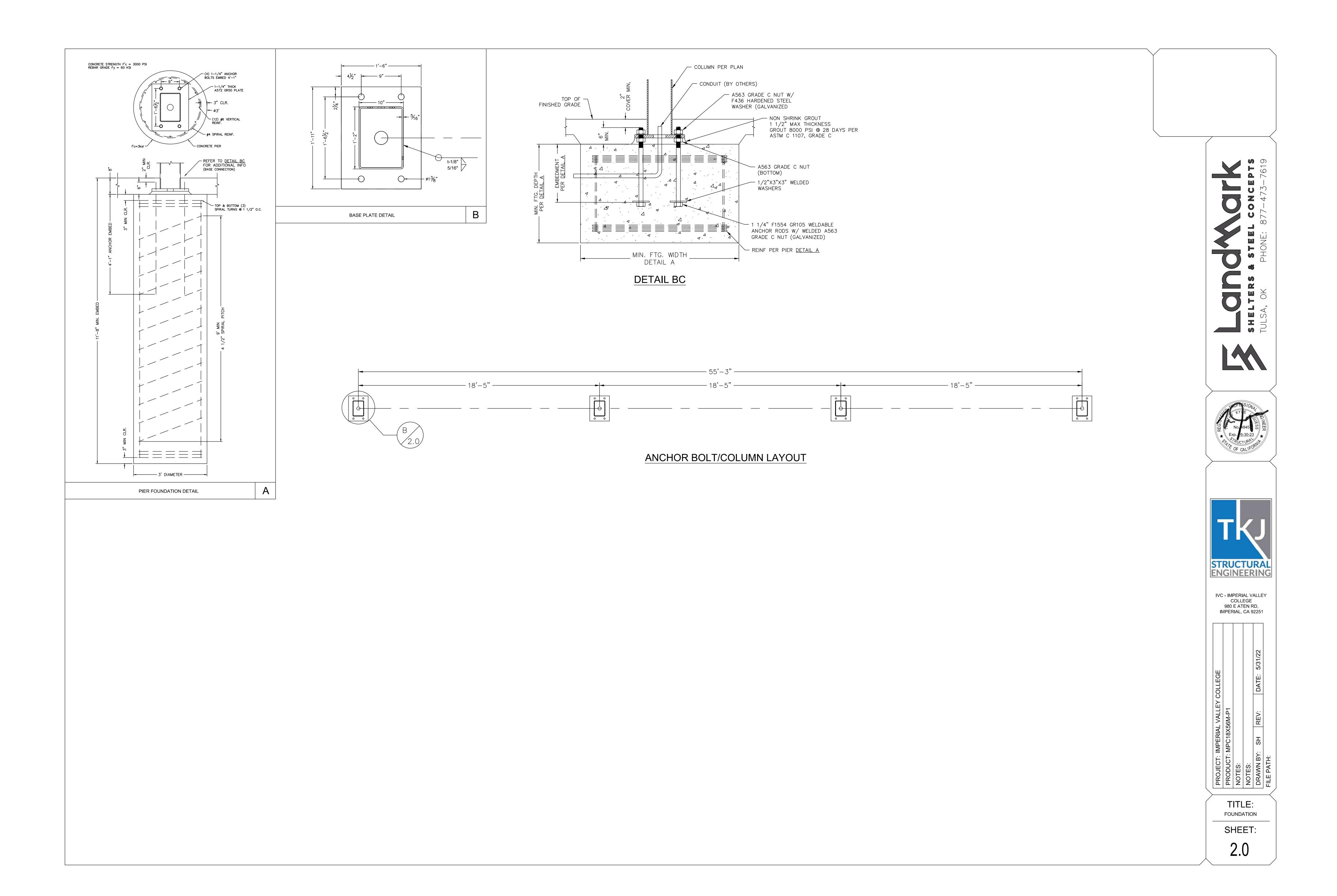
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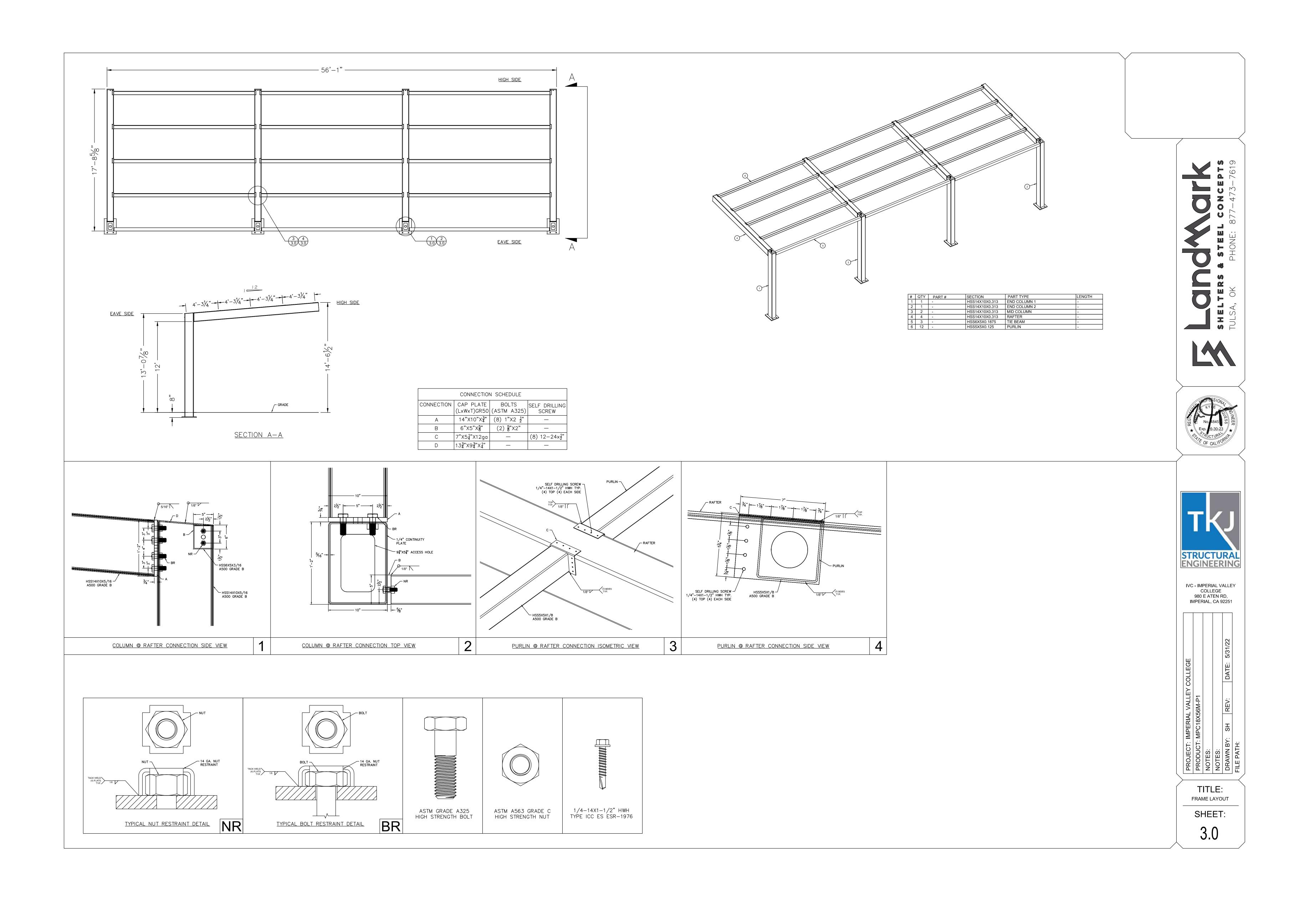
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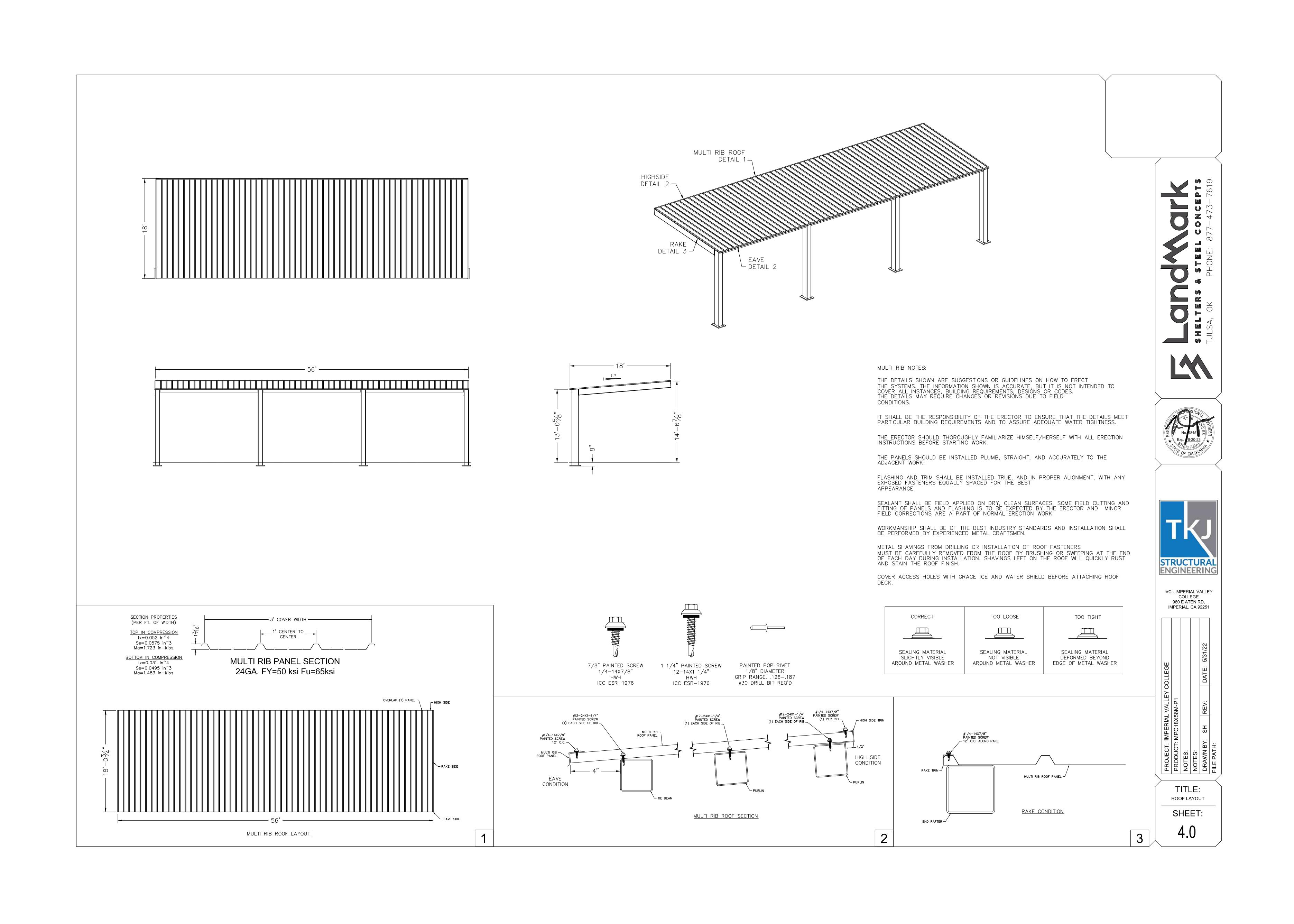
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COVER PAGE

SHEET:







GENERAL

- 1. SPECIFIC NOTES & DETAILS ON THE SHADE STRUCTURE STRUCTURAL DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES & TYPICAL DETAILS
- WHERE NO DETAILS ARE SHOWN OR NOTED IN ANY PART OF THE WORK THE DETAILS FOR OTHER SIMILAR WORK SHALL APPLY.
- 3. DETAILS IDENTIFIED AS TYPICAL, SHALL APPLY IN ESTIMATING AND CONSTRUCTION TO EVERY LIKE CONDITION WHETHER OR NOT THE
- 4. THE STRUCTURAL DRAWINGS SHALL NOT BE SCALED. COORDINATE DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- 5. COORDINATE ELEVATIONS, SLOPES AND DRAINAGE REQUIREMENTS WITH THE ARCHITECTURAL DRAWINGS.
- 6. STANDARDS REFERENCED ON THE STRUCTURAL DRAWINGS REFER TO THE EDITION APPLICABLE UNDER THE CURRENT BUILDING CODE.
- 7. THE RESPONSIBILITY FOR THE REVIEW AND COORDINATION OF DRAWINGS AND SPECIFICATIONS PRIOR TO THE START OF RELATED CONSTRUCTION SHALL BEAR ON THE CONTRACTOR. DISCREPANCIES THAT EXIST SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER IN A TIMELY MANNER, PRIOR TO START OF RELATED CONSTRUCTION.

WORK PERFORMED IN CONFLICT WITH THE STRUCTURAL DRAWINGS

- OR APPLICABLE BUILDING CODE REQUIREMENTS SHALL BE CORRECTED AT THE EXPENSE OF THE CONTRACTOR.
- DIMENSIONS, ELEVATIONS, AND SITE CONDITIONS SHALL BE VERIFIED BEFORE STARTING RELATED WORK AND THE ENGINEER
- NOTIFIED OF DISCREPANCIES IN A TIMELY MANNER. 10. SITE CONDITIONS THAT ARE NOT REFLECTED ON THE STRUCTURAL DRAWINGS OR THAT DEVIATE FROM THE MAXIMUM OR MINIMUM DIMENSIONS INDICATED SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER IN A TIMELY MANNER. SUCH CONDITIONS MAY INCLUDE CONFLICT IN GRADES, ADVERSE SOIL CONDITIONS,
- GROUND WATER PRESENT, DEEPENED FOOTINGS, UNCOVERED AND UNEXPECTED UTILITY LINES, ETC. 11. MATERIALS AND WORKMANSHIP SHALL CONFORM TO

FOUNDATION

REQUIREMENTS OF APPLICABLE REGULATIONS AND THE BUILDING

CODE AS AMENDED AND ADOPTED BY THE BUILDING OFFICIAL.

THE STRUCTURE WILL BE LOCATED ENTIRELY ON UNDISTURBED NATIVE SOIL. IF THE BUILDING INSPECTOR SUSPECTS FILL, EXPANSIVE SOILS OR ANY GEOLOGIC INSTABILITY BASED UPON OBSERVATION OF THE FOUNDATION EXCAVATION, A SOILS OR GEOLOGICAL REPORT, AND RESUBMITTAL OF PLANS TO PLAN CHECK TO VERIFY THAT THE REPORT RECOMMENDATIONS HAVE BEEN INCORPORATED, MAY BE REQUIRED.



- THE MAXIMUM ALLOWABLE SOIL BEARING PRESSURE SHALL BE 1500 psf. THE RESULTING ALLOWABLE BEARING VALUE MAY BE INCREASED BY 1/3 FOR WIND AND SEISMIC LOAD CASES.
- FOOTING DEPTHS INDICATED ON THE STRUCTURAL DRAWINGS ARE FOR BIDDING PURPOSES ONLY AND ARE ASSUMED TO BE IN SUITABLE BEARING MATERIALS.
- FOOTING ELEVATIONS SHALL BE LOCATED SUCH THAT THEIR BEARING IS A MINIMUM HORIZONTAL DISTANCE OF 10 FEET FROM THE DAYLIGHT OF AN ADJACENT SLOPE.
- ANCHOR BOLTS, DOWELS AND HOLD-DOWN ANCHORS SHALL BE TIED IN PLACE PRIOR TO FOUNDATION INSPECTION.

SUBMITTALS

- 1. THE CONTRACTOR SHALL MAKE SUBMITTALS PRIOR TO FABRICATION AS REQUIRED BY THE WRITTEN SPECIFICATIONS AND SHALL INCLUDE AS A MINIMUM THE FOLLOWING SUBMITTALS:
- CONCRETE MIX DESIGNS
- REINFORCING STEEL DRAWINGS STRUCTURAL STEEL DRAWINGS
- METAL DECK DRAWINGS WELDING PROCEDURES (SHOP AND FIELD WELDING).
- 2 THE FOLLOWING SHOP DRAWINGS ARE NOT REQUIRED FOR SUBMISSION FOR STRUCTURAL REVIEW:
- SHORING AND BRACING UNSPLICED REBAR AT SLAB-ON-GRADE AND FOOTINGS
- FORMWORK D. STRUCTURAL STEEL MILL REPORTS
- STEEL REINFORCING LISTS AND QUANTITIES AND LENGTHS OF ALL MATERIALS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO ASSURE COMPLIANCE WITH THE PLANS. ENGINEER WILL NOT
- SHOP DRAWINGS SUBMITTED TO THE ENGINEER FOR REVIEW SHALL BE STAMPED AND SIGNED BY THE CONTRACTOR INDICATING THE CONTRACTORS PRIOR REVIEW AND THAT THE SUBMITTAL IS IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- AS AN ALTERNATE TO MULTIPLE PAPER COPIES, SHOP DRAWINGS MAY BE SUBMITTED IN ELECTRONIC (PDF) FORMAT. WHERE SUBMITTED ELECTRONICALLY, SHOP DRAWINGS WILL BE PROCESSED AND RETURNED ELECTRONICALLY.

REINFORCING STEEL

- DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS SHALL FOLLOW ACI 315, "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT."
- 2. REINFORCING BARS SHALL CONFORM TO ASTM A 615, GRADE 60,
- 3. WELDED REINFORCING BARS SHALL CONFORM TO ASTM A 706, GRADE 60. BARS TO BE WELDED CONFORMING TO ASTM A 615, GRADE 60 MAY BE SUBSTITUTED PROVIDED THAT THE WELDING OF BARS COMPLIES WITH AWS D1.4 AND THE MINIMUM SPECIFICATIONS FOR WELDING OF REINFORCING STEEL INCLUDED HEREIN.
- 4. REINFORCING BAR LAP SPLICES SHALL BE CLASS B. (18" MIN.) FOR CONCRETE, U.O.N.

6. REINFORCING BARS FOR CONCRETE SHALL BE PROVIDED WITH THE

- 5. DETAILS OF REINFORCEMENT SHALL COMPLY WITH ACI 318, CHAPTER 25.
- FOLLOWING MINIMUM COVER: CONC. CAST AGAINST EARTH FORMED CONC. EXPOSED TO EARTH/WEATHER: #5 OR SMALLER #6 OR LARGER SLABS (#11 AND SMALLER)

BEAMS & GIRDERS

- 7. #3 SPACER TIES SHALL BE INSTALLED AT 30" ON CENTER IN ALL BEAMS AND FOOTINGS TO SECURE REINFORCING BARS IN PLACE,
- 8. AT THE CONTRACTOR'S OPTION, INTERNALLY-THREADED LENTON A2 COUPLERS (IAPMO UES-0129) MAY BE USED IN LIEU OF LAP SPLICES. MECHANICAL BAR SPLICES MAY BE TYPE 1 OR TYPE 2 AS DEFINED IN

REINFORCED CONCRETE

- CONCRETE CONSTRUCTION SHALL CONFORM WITH CHAPTER 19A OF THE BUILDING CODE AND TO THE PROVISIONS OF ACI 318, LATEST EDITION.
- READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH ASTM C 94.
- CEMENT SHALL CONFORM TO ASTM C 150 TYPE I OR II. LOW ALKALI, OR TYPE V FOR SULFATE EXPOSURE CLASS S2 AND S3.
- AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL
- CONFORM TO ASTM C 33.
- 5. NORMAL WEIGHT CONCRETE SHALL HAVE A MAXIMUM DRY DENSITY OF 150 PCF. 6. CONCRETE MIXES SHALL BE DESIGNED BY A QUALIFIED TESTING LABORATORY AND APPROVED BY THE ENGINEER. MIX

DESIGN METHODS (TEST HISTORY OR TRIAL BATCH METHOD) IN

ACCORDANCE WITH ACI 318, SECTION 26.4 SHALL BE USED TO

PROPORTION CONCRETE. MINIMUM CONCRETE COMPRESSIVE STRENGTHS AT 28 DAYS, MAXIMUM SLUMPS, AND MAXIMUM WATER/CEMENT RATIOS SHALL BE AS FOLLOWS:

<u>DESCRIPTION</u>	MIN 28 DAY fc	<u>SLUMP</u>	MAX <u>R/</u>
FOOTINGS	3000 PSI	4"+/-1'	0.:
ALL OTHER CONCRETE	3000 PSI	4"+/-1"	0.:

- 8. ADMIXTURES SHALL BE APPROVED IN ADVANCE.
- 9. SLUMPS INDICATED ARE PRIOR TO PLASTICIZER ADDITIVES.
- 10. CONCRETE ADMIXTURES CONTAINING CHLORIDE OR CHLORIDE SALTS SHALL NOT BE USED EXCEPT WHERE APPROVED IN WRITING BY THE ENGINEER.
- 11. FLYASH SHALL BE LIMITED TO NO MORE THAN 20% OF THE TOTAL WEIGHT OF CEMENTITIOUS MATERIALS IN THE CONCRETE, U.O.N.
- 12. WATER MAY BE ADDED ON SITE TO OBTAIN SPECIFIED SLUMPS PROVIDED THAT IT IS ADDED WITHIN ONE HOUR OF BATCHING AND SITE-ADDED WATER IS SPECIFIED ON THE BATCH REPORT. SITE-ADDED WATER SHALL NOT COMPROMISE THE STRENGTH OR SLUMP OF THE CONCRETE.
- 13. CONCRETE SHALL NOT BE PLACED BEYOND 1-1/2 HOURS FOLLOWING BATCHING.
- 14. CONDUIT OR PIPES LARGER THAN 4" NOMINAL DIAMETER SHALL NOT BE PLACED IN CONCRETE. SLEEVES FOR OPENINGS IN CONCRETE SHALL BE INSTALLED BEFORE PLACING REINFORCING WHICH MAY CONFLICT SHALL NOT BE CUT UNLESS APPROVED IN WRITING BY THE ENGINEER.
- 15. STEEL COLUMNS, BEAMS PURLINS & PANELS MAY BE ERECTED 24 HOURS AFTER FOUNDATION CONCRETE PLACEMENT OR AFTER CONCRETE REACHES A COMPRESSIVE STRESS OF 1000 PSI, WHICHEVER COMES FIRST.

STRUCTURAL STEEL

- 1. STRUCTURAL STEEL WORK SHALL BE PERFORMED IN ACCORDANCE WITH CHAPTER 22 OF THE BUILDING CODE, AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" AND AISC 303 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND
- 2. STRUCTURAL STEEL STRENGTHS AND GRADES SHALL BE AS FOLLOWS, U.O.N.:

<u>DESCRIPTION</u> ANGLES, CHANNELS	<u>YIELD</u> Fy,ksi	ASTM GRADE
& OTHER SHAPES	36ksi	A36
HSS SECTIONS	46ksi	A500, GR B
CONNECTION PLATES & MISC., U.O.N.	50ksi	A572 GR 50

- 3. ANCHOR RODS SHALL CONFORM TO ASTM F 1554, GRADE 105, UNLESS OTHERWISE NOTED. NUTS FOR ANCHOR RODS SHALL CONFORM TO ASTM A 563, GRADE C HEX (HEAVY HEX WHERE ANCHOR ROD DIAMETER IS GREATER THAN 1 1/2").
- 4. MAIN MEMBER SHALL HAVE HIGH STRENGTH BOLTS CONFORMING TO AISC SPECS FOR ASTM A 325N BOLTS, U.O.N. OTHER BOLTS SHALL CONFORM TO ASTM A 307. NUTS FOR HIGH STRENGTH BOLTS SHALL BE HEAVY HEX GRADE C CONFORMING TO ASTM
- 5. EXTERIOR STRUCTURAL STEEL PERMANENTLY EXPOSED TO THE WEATHER SHALL BE HOT-DIP GALVANIZED AFTER FABRICATION. ZINC COATING SHALL CONFORM TO ASTM A123 (G60, U.O.N.).
- 6. TIGHTEN ASTM A 325N BOLTS TO "SNUG-TIGHT" CONDITION PER AISC SPECIFICATION FOR STRUCTURAL JOINTS. TEST ASTM A 325SC BOLTS WITH A CALIBRATED WRENCH UNLESS LOAD INDICATOR BOLTS ARE USED.
- WELDING DONE AFTER GALVANIZING SHALL BE PROTECTED ACCORDING TO ASTM A780.
- PROVIDE BEVELED WASHERS PER ANSI B18.23.1 AS REQUIRED ON SLOPED SURFACES.
- 9 FAYING SURFACES OF HOT-DIPPED GALVANIZED MEMBERS AND CONNECTORS SHALL BE ROUGHENED WITH A HAND WIRE-BRUSH PRIOR TO ERECTION. ROUGHENING PROCESS SHALL VISIBLY ALTER THE GALVANIZED SURFACE WITHOUT DISRUPTING THE CONTINUITY OF GALVANIZATION. POWER WIRE-BRUSHING NOT ALLOWED.

WELDING

- 1. WELDING SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF THE AMERICAN WELDING SOCIETY (AWS D1.1).
- 2. WELDING OF METAL DECK AND LIGHT GAUGE METAL FRAMING SHALL BE DONE BY CERTIFIED LIGHT GAUGE WELDERS IN ACCORDANCE WITH AWS SPECIFICATIONS FOR WELDING SHEET STEEL IN STRUCTURES, AWS D1.3.
- 3. WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS.
- 4. WELDING ELECTRODE FOR THE SHIELDED METAL-ARC WELDING (S.M.A.W.) PROCESS AND WELDING ELECTRODE SHALL CONFORM TO AWS A5.1 "SPECIFICATION FOR CARBON STEEL ELECTRODES FOR SHIELDED METAL ARC WELDING."
- ELECTRODES FILLER MATERIAL SHALL BE A MINIMUM OF E70XX U.O.N., EXCEPT E60XX MAY BE USED FOR WELDING OF METAL DECK AND LIGHT GAUGE FRAMING.
- 6. WELDS SHALL HAVE WELD CONTROLLED SEQUENCE AND TECHNIQUE IN ORDER TO MINIMIZE SHRINKAGE STRESSES AND DISTORTION.

METAL DECK

METAL DECK SHALL BE OF THE TYPE AND GAUGE AS INDICATED ON THE DRAWINGS AND SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND PUBLISHED ICC REPORT.

METAL DECK AND ACCESSORIES SHALL BE FORMED FROM STEEL

- SHEETS CONFORMING TO ASTM A 653, WITH COATING DESIGNATION G60 [G90].
- 3. DECK SHALL BE CONTINUOUS OVER THREE SPANS WHEREVER POSSIBLE. SHORE DECK IF RECOMMENDED BY THE MANUFACTURER. MINIMUM BEARING AT ENDS IS 2".

4. WHERE 3/4" DIAMETER SHEAR STUDS ARE TO BE WELDED TO

5. CONTRACTOR SHALL CUT DECK PER STRUCTURAL DETAILS AT ALL OPENINGS, COLUMNS, AND REQUIRED PENETRATIONS AND SHALL SUPPLY NECESSARY ACCESSORY ITEMS SUCH AS CLOSURES, CLIPS, ETC.

SUPPORTS, 18 GAUGE (OR THICKER) DECKING SHALL NOT BE

SHEET METAL SCREWS

- SHALL BE INSTALLED PER THE MANUFACTURER'S
- 2. SCREWS TO CONFORM TO THE REQUIREMENTS OF ASTM C1513.

3. UNLESS OTHERWISE NOTED, SCREWS SHALL CONFORM TO THE

A. B. C.	SIZE: #12 ZINC-PLATED HARDENED ST SELF-TAPPING HEX-HEAD
D.	HEX-HEAD

TOGETHER SNUGLY.

- PENETRATE BOTH PLIES OF MATERIAL TO BE ATTACHED, WITH A MINIMUM OF 3 THREADS OF SCREW TIP EXPOSED.
- 5. SCREWS SHALL BE TIGHTENED IN SHUCH A MANNER THAT THE WASHER-FACE OF SCREW HEAD IS FLUSH WITH ADJACENT PLY,
- 6. SCREWS SHALL MEET THE FOLLOWING MINIMUM STRUCTURAL

THINNER OF PLIES	ALLOWABLE	ALLOWABLE
TO BE JOINED	SHEAR	PULL-OUT
20 GA.	200#	99#
18 GA.	308#	132#
16 GA.	430#	165#
14 GA.	601#	207#
12 GA.	724#	289#

DESIGN CRITERIA

DESCRIPTION

LONG PERIOD SITE COEFFICIENT, FV

HORIZONTAL OR VERTICAL IRREGULARITIES TYPE (S)

NUMBER OF STORIES

- SCREWS SHALL BE ITW BUILDEX "TEKS" PER ICC ESR-1976 AND RECOMMENDATIONS.

FOLLOWING REQUIREMENTS:

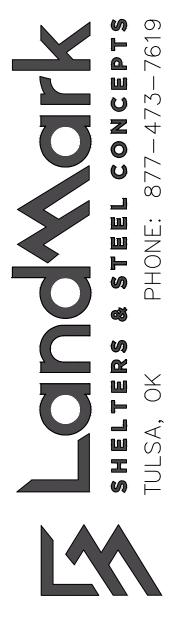
- DURING INSTALLATION OF SCREWS, THE SCREW SHALL
- AND PLIES OF MATERIAL TO BE FASTENED ARE DRAWN

DEAD & LIVE LOADS	
ROOF LIVE LOAD	20 PSF
ROOF PANEL DEAD LOAD	1.1 PSF
WIND DESIGN	
BASIC WIND SPEED (3 SECOND GUST), Vult	98 MPH
RISK CATEGORY	II
EXPOSURE CATEGORY	С
FACTORS: Kz, KzT (1 MINIMUM) Kd	0.85 / 1.0 / 0.85
qh = 0.00256 Kz KzT Kd V2	17.74 PSF
CLEAR WIND FLOW	[] NO [X] YES
OBSTRUCTED WIND FLOW	[] NO [X] YES
SEISMIC DESIGN	
LATERAL FORCE-RESISTING SYSTEM	STEEL - ORDINARY CANTILEVER COLUMN
ANYLYSIS PROCEDURE	EQUIVALENT LATERAL FORCE
SEISMIC DESIGN CATEGORY (SDC)	E
SEISMIC SITE CLASS	D
SEISMIC IMPORTANCE FACTOR, le	1
DESIGN BASE SHEAR, V	Cs x W = 1.10 X 1.414 = 1.56 PSF
SEISMIC RESPONSE COEFFICIENT, Cs	1.414
RESPONSE MODIFICATION FACTOR, R	1.25
OVERSTRENGTH FACTOR, Ω	1.25
SHORT-PERIOD SITE COEFFICIENT, Fa	1.2
DESIGN SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD, Sds-USED TO DETERMINE Cs (WITH CAP PER CBC 1616.10.00)	1.767
MAPPED SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD, S1	0.786

DESIGN VALUES

ALLOWABLE SHAFT RESISTANCE FOR FOUNDATIONS CBC TABLE 1806A.2 FLOOD DESIGN - DESIGN ASSUMED TO NOT BE IN FLOOD HAZARD AREA IF PROJECT IS LOCATED IN A FLOOD ZONE OTHER THAN ZONE X, A LETTER STAMPED & SIGNED FROM A SOILS ENGINEER IS REQUIRED	
TO VALIDATE THE ALLOWABLE SOIL VALUES SPECIFIED ARCHITECTURAL REQUIRE	
DESCRIPTION	DESIGN VALUES
TYPE OF CONSTRUCTION	II-B
OCCUPANCY CLASSIFICATION	A-3

1.70







IVC - IMPERIAL VALLEY

COLLEGE

980 E ATEN RD, IMPERIAL, CA 92251

COVER PAGE

SHEET:

